EMERGENCY ACTION PLAN FOR PETIT LAKE DAM

STATE ID NO. 112-009-00462; NID NO. GA00685 Revision 3



PETIT LAKE – BIG CANOE PICKENS COUNTY, GEORGIA



Submitted to

BIG CANOE PROPERTY OWNER'S ASSOCIATION

10586 Big Canoe Jasper, Georgia 30143

Submitted by

May 2018



ongineers | scientists: | innovators





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1. INTRODUCTION

1.1 Statement of Purpose

The purpose of the Emergency Action Plan (EAP) is to establish procedures for warning, evacuating and protecting persons and property which would be endangered in the event of a failure of the Petit Lake Dam, in Big Canoe, Georgia, as well as taking timely action to notify the appropriate emergency management agencies, law enforcement bodies, and/or governing officials of possible, impending, or actual failure of the dam.

1.2 History of Document

The EAP for Petit Lake Dam was originally created and revised by another consultant (Jordan, Jones, and Goulding) in 1998 and 2007, respectively. A summary of known dates of revision of this EAP is provided in Appendix E and future revisions and modifications should be added to this table.

This EAP was revised by Geosyntec Consultants, Inc. (Geosyntec), and relies upon information and analyses developed by others as identified in the Plan. Geosyntec has developed this plan in good faith for use by the Big Canoe Property Owner's Association (POA) and the Emergency Management Agencies responsible for Petit Lake Dam. This plan may not be relied upon by other parties and may not be modified without the express written permission of Geosyntec.

Geosyntec updated the EAP for Petit Lake Dam based on guidance from the Georgia Safe Dams Program document *Engineer Guidelines* (2015), and templates referred to in *Engineer Guidelines* created by the North Carolina Department of Environmental Quality (2016) and the Association of State Dam Safety Officials (2010).

The inundation mapping and calculations provided in Appendix B were original created by Jordan, Jones, and Goulding and have not been updated by Geosyntec. The Figures provided of inundation maps and Big Canoe property parcels, and the names and addresses provided in Tables 1 and 2 were provided by Big Canoe. The property parcels and addresses for properties downstream of Big Canoe in Table 3 were provided by the Pickens County Emergency Management Agency (EMA).

2. NOTIFICATION FLOWCHART

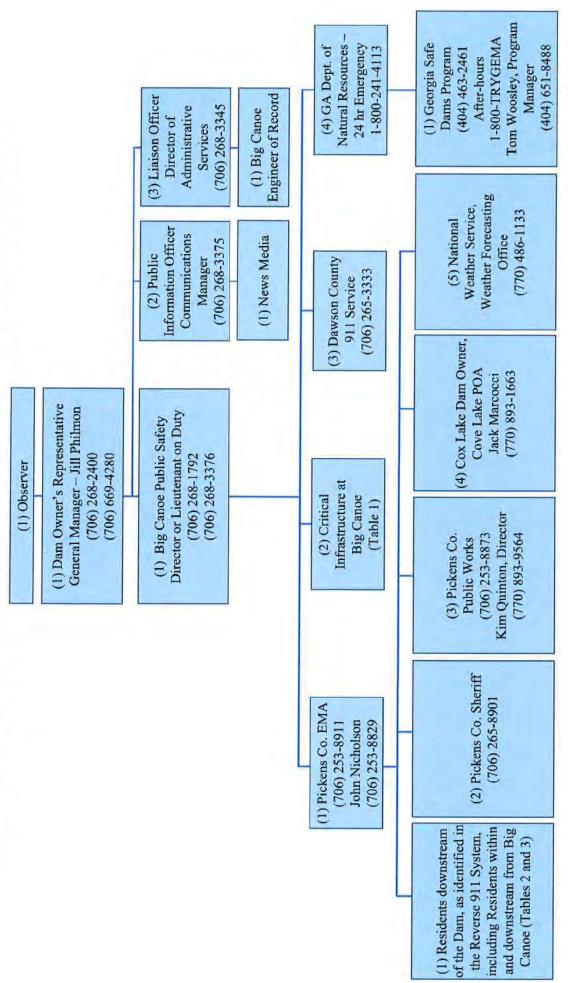
The Notification Flowcharts (Figure 1 and 2) summarize the following information which is applicable during an impending or imminent failure of the Petit Lake Dam:

- Who is responsible for notifying Petit Lake Dam owner representative(s) and/or emergency management official(s);
- Who is to be notified; and
- What is the priority order in which individuals are to be notified.

All residents and employees of Big Canoe can and should be observers of unusual events at Petit Lake Dam. This observer group will be educated through the local newsletter regarding what are symptoms of impeding or imminent failure, and who at the POA should be contacted to initiate the Notification Process.

The Dam Owner's Representative or his designee is responsible for initiating the Notification Process. The Dam Owner's Representative or his designee will verify the condition of Petit Lake Dam which the observer has identified and initiate the notification. The Dam Failure Notification Flow Charts (Figures 1 and 2 and Tables 1, 2, and 3) identify the residents, emergency management agencies, government agencies and Big Canoe Property Owners Association that should be contacted immediately and in what order. Subsequent contacts by each of these individuals are shown in priority order.

Dam Failure Notification Flowchart for Conditions C (Level 1) & B (Level 2) Emergencies

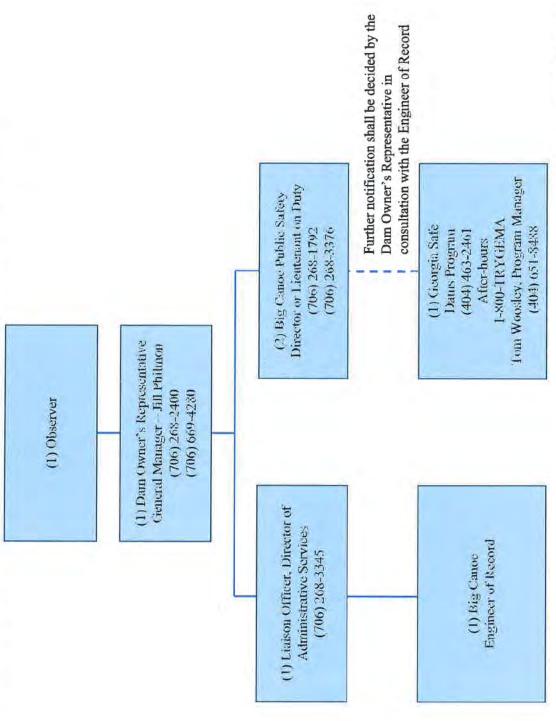


(#) = Order of Priority of Notification

Note: Flowchart and contact information provided by Big Canoe Property Owner's Association on 9, 19 and 19 June 2017.

Figure 2: Petit Lake Dam

Dam Failure Notification Flowchart for Condition A (Level 3) Emergencies



(#) = Order of Priority of Notification

Note: Flowchart and contact information provided by Big Canoe Property Owner's Association.

TABLE 1⁽¹⁾ DAM FAILURE NOTIFICATION OF BIG CANOE CRITICAL INFRASTRUCTURE JUST BELOW THE DAM

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

FACILITY	ADDRESS/LOT NUMBER	PHONE NUMBER
Utilities Office	Highland Trail/Wolfscratch Drive	706/268-3400
Swim Club	Wolfscratch Drive	706/268-3317
Fitness Center	Wolfscratch Drive	706/268-3441
Chapel	Wolfscratch Drive	706/268-3203
Tennis Center	Wolfscratch Drive	706/268-3367
IGA Express	Wolfscratch	706/268-3326
Lakewatch Village	Golf Club Drive	706/268-3326
Clubhouse	Golf Club Drive	706/268-3326
Fire Station #3	Wolfscratch Drive	706/268-1792
Duffers	Golf Club Drive	706/268-3273
Golf Shop	Golf Club Drive	706/268-3323
Cart Barn	Golf Club Drive	706/268-3323

Note:

⁽¹⁾ Facilities and contact information provided by Big Canoe Property Owner's Association on 26 April 2017 based on Inundation Mapping conducted by Jones, Jordan, and Goulding

TABLE $2^{\rm th}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

	*	Street Name Address	Owner's Name	Email address 1	Email address 2 BC Home 1	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
Wilderness Parkway Tanager Way Indigo Bunting Trail Bluebird Court Canada Geese Point Savallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	2569	Wildemess Parkway	Campbell, Douglas &						
Tanager Way Indigo Bunting Trail Bluebird Court Canada Geese Point Swallow Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Holly Point Thrush Turn Thrush Turn Holly Point	5888	Wildemess Parkway	Williams, Sally						
Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Canada Geese Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Holly Point Thrush Turn Thrush Turn Thrush Turn Thrush Turn Thrush Turn Thrush Turn	87	Tanager Way	Farias, Fran & Phillip						
Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petir Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	216	Indigo Bunting Trail	Tumer, Jeffrey						
Indigo Bunting Trail Indigo Bunting Trail Indigo Bunting Trail Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Canada Geese Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Holly Point Thrush Turn	254	Indigo Bunting Trail	Натет, David						
Indigo Bunting Trail Indigo Bunting Trail Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	270	Indigo Bunting Trail	Risoldi, Ciro						
Indigo Bunting Trail Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Swallow Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Holly Point Thrush Turn Thrush Turn Holly Point	288	Indigo Bunting Trail	Honeycutt, Boyce						
Indigo Bunting Trail Bluebird Court Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	289	Indigo Bunting Trail	Lacey, Claude						
Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Holly Point Willow Drive	255	Indigo Bunting Trail	Rozeboom, Loren						
Bluebird Court Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petir Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	0	Bluebird Court	Reynolds, David & Patsy						
Canada Geese Point Canada Geese Point Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Thrush Turn	18	Bluebird Court	Reynolds, David & Patsy						
Canada Geese Point Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn	172	Canada Geese Point	Pepper, Ralph						
Canada Geese Point Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Thrush Turn	230	Canada Geese Point	Ebbeskotte, Carolyn						
Goldfinch Point Petit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn	205	Canada Geese Point	May, Randy						
Feit Ridge Drive Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn	121	Goldfinch Point	Perkel, Karen and Ralph						
Flycatcher Point Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Thrush Turn Thrush Turn Thrush Turn Willow Drive	36	Petit Ridge Drive	Huffstetler, Mark & Rachel						
Swallow Point Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Thrush Turn Thrush Turn Willow Drive	48	Flycatcher Point	Clough, Gerald & Anne						
Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Thrush Turn Willow Drive	190	Swallow Point	Leff, Steven						
Swallow Point Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	181	Swallow Point	Malgeri, Linda						
Swallow Point Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	115	Swallow Point	Arthurs, Lee						
Swallow Point Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	95	Swallow Point	Kelly, David and Laura						
Thrush Turn Thrush Turn Thrush Turn Holly Point Willow Drive	0	Swallow Point	Bradley Family Trust						
Thrush Turn Thrush Turn Holly Point Willow Drive	18	Thrush Turn	Coleman, William						
Thrush Turn Holly Point Willow Drive	56	Thrush Тит	Reisinger, Ralf & Daniela						
Holly Point Willow Drive	74	Thrush Turn	#47 Holding LLC						
Willow Drive	40	Holly Point	Young, Alden						
	287	Willow Drive	Roberson, Ellen						

TABLE $2^{(i)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

135 135 145 145 145	Chestnut Rise Chestnut Rise Chestnut Rise Chestnut Rise	Collins, Norma Abemathy, MaryAnn			
135 135 145 145	Chestnut Rise Chestnut Rise Chestnut Rise	Abemathy, MaryAnn			
135 145 145	Chestnut Rise Chestnut Rise	Court Mosesson P. Louise			
145	Chestnut Rise	эсоц, поппап ос лоппу			
145		Milles, Purtell			
145	Chestnut Rise	Faust, Richard & Wendy			
	Chestnut Rise	Moser, Nancy			
145	Chestnut Rise	Ferrini, Joseph			
155	Chestnut Rise	House, Jerry			
155	Chestnut Rise	Eaves, Dennis			
155	Chestnut Rise	Doll, Herman			
20	Sconti Ridge	Manning, Joyce			
20	Sconti Ridge	Parks, William			
20	Sconti Ridge	Collier, Daryl & Linda			
20	Sconti Ridge	Hanson/Shaw			
34	Sconti Ridge	Brown, Donald & MaryAnn			
34	Sconti Ridge	Hayes, Jr., Mark			
34	Sconti Ridge	Young, Shelia			
34	Sconti Ridge	Verell, William & Susan			
48	Sconti Ridge	Wagner, William & Patricia			
8	Sconti Ridge	Sherman, Karen			
48	Sconti Ridge	Hacker & Ramscy			
98	Sconti Ridge	Campbell,Mark			
86	Sconti Ridge	McCann, Debra and Ronald			
98	Sconti Ridge	Captan, Mohamed & Marcia			
98	Sconti Ridge	Wood, Barbara			
116	Sconti Ridge	Mullins, Jr. B.A.			
116	Disharoon	Floyd, Greg			

TABLE $2^{(l)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
116	Sconti Ridge	Capone & Rutledge						
140	Sconti Ridge	Riley, Ronald						
40	Treetop Lane	Chait, Susan						
104	Treetop Lane	Paynter, Fredrick						
309	Buckskull Hollow Drive	Brown, Carolyn						
301	Buckskull Hollow Drive	Warren, Claude						
57	Buckskull Hollow Drive	Henil & Brennan						
41	Buckskull Hollow Drive	Dollat, Dari & David						
11	Buckskull Hollow Drive	Bassett, Robert & Jolyne						
22	Buckskull Hollow	McCardel, James & Dora Jean						
56	Buckskull Hollow	Davis, James						
84	Highland Court	Nicholson						
110	Highland Court	Lundstrom						
0	Highland Court	BCLLC						
196	Sconti Knoll Drive	Cody, Gail						
200	Sconti Knoll Drive	Horsley, Rosemarie						
193	Sconti Knoll Drive	Kowalski, George						
191	Sconti Knoll Drive	Tanner, Caroll						
09	Sconti Ridge	Hackney						
98	Sconti Point	Crawford, Howard Wayne						
0	Sconti Point	Rhodus, Gary and Marsha						
0	Sconti Point	Bhold, Adriana						
0	Sconti Point	Bhold, Adriana						
62	Sconti View Drive	Slade, David						
34	Sconti Ct.	Landreth, Porter						
21	Chestnut Knoll Point	Rees, James						
5	Chestnut Knoll Point	Moon, Jacqueline						

TABLE $2^{(i)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

Chemina Kioli Posis Bowen, Jouane Cottonia Kioli Posis Learnin, Paulo Chemina Kioli Posis Canuarin, Paulo Chemina Kioli Canuarin, Paulo Chemina Paulo Chemina Minima Kioli Canuarin, Paulo Chemina Paulo Chemina Minima Kioli Canuarin, Paulo C	*	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
Chestnut Knoll Point Chestnut Knoll Point Chestnut Knoll Chestnut Knoll Chestnut Knoll Chestnut Knoll Tanager Way Wolfscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Brive Wilderness Parkway	7	Chestnut Rise	Bowers, Joanne						
Chestnut Knoll Point Chestnut Knoll Point Chestnut Knoll Point Tanager Way Wolfscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Drive Clubhouse Brives Wilderness Parkway	9	Chestnut Knoll Point	Leardini, Paolo						
Chestnut Knoll Chestnut Knoll Tanager Way Wolfscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Drive Wilderness Parkway	11	Chestnut Knoll Point	Gurusamy & Subbannan						
Chestnut Knoll Point Tanager Way Wolfscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Drive Wilderness Parkway	31	Chestnut Knoll	Genone, Тетту and Loretta						
Tanager Way Wolfscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Drive Clubhouse Drive Clubhouse Brives Wilderness Parkway	41	Chestnut Knoll Point	Nelson, Ronald						
Woifscratch Village Cir. Clubhouse Drive Clubhouse Drive Clubhouse Drive Wilderness Parkway	28	Tanager Way	Beraha, Dan & Ellen						
Clubhouse Drive Clubhouse Drive Clubhouse Drive Wilderness PKWY Clubhouse Drive Wilderness Parkway	226	Wolfscratch Village Cir.	Chapel						
Clubhouse Drive Clubhouse Drive Wilderness PKWY Clubhouse Drive Clubhouse Brikway Wilderness Parkway	51	Clubhouse Drive	Apgar, Cindy & Dan						
Clubhouse Drive Wilderness PKWY Clubhouse Drive Clubhouse Drive Wilderness Parkway	79	Clubhouse Drive	Fargason, Charles & Gail						
Wilderness PKWY Clubhouse Drive Clubhouse Drive Wilderness Parkway	65	Clubhouse Drive	Dubose Calvin Ir. & Mary ann						
Clubhouse Drive Clubhouse Drive Wildemess Parkway	1944	Wilderness PKWY	Mcclure, William & Nancy						
Clubhouse Drive Wilderness Parkway		Clubhouse Drive	Bowers-Mcclure, Mark & Joanne						
Wildemess Parkway	29	Clubhouse Drive	Bell, Pamela & Кету						
Wilderness Parkway	1944	Wildeness Parkway	Hecht, Bruce and Rosemary						
Wildemess Parkway	1944	Wilderness Parkway	Tillery, Roger & Patty						
Wildemess Parkway Choctaw Pass	1944	Wildemess Parkway	Prince, Bruce & Judith		G				
Wildemess Parkway Choctaw Pass	1944	Wilderness Parkway	Chu, Kathleen and Norman						
	1944	Wildemess Parkway	Gregg, Fredrick & Sue						
Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Choctaw Pass	1944	Wildemess Parkway	Kadechuk, Ватту & Linda						
Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Choctaw Pass	1944	Wilderness Parkway	Delashmutt, R. E.						
Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Wildemess Parkway Choctaw Pass	1944	Wilderness Parkway	Hyland, Keith and Lauren						
Wildemess Parkway Wildemess Parkway Wildemess Parkway Chociaw Pass	1944	Wildemess Parkway	Lehmberg, Bill & Peggy						
Wildemess Parkway Wildemess Parkway Wildemess Parkway Choctaw Pass	1944	Wilderness Parkway	Zamborsky, Joe and Lis						
Wildemess Parkway Wildemess Parkway Choctaw Pass	1944	Wilderness Parkway	Salcedo, Cesar & Anna						
Wildemess Parkway Choctaw Pass	1944	Wildemess Parkway	Dicus, Patti & Frank						
Choctaw Pass	1944	Wildemess Parkway	Farias, Fran & Phillip						
	315	Choctaw Pass	Anne Bopp						

TABLE $\mathbf{2}^{(0)}$ DAM FAILURE NOTIFICATION OF BIG CANOE RESIDENTS AND OTHER FACILITIES

EMERGENCY ACTION PLAN FOR PETIT LAKE BAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Owner's Name	Email address 1	Email address 2	BC Home Number	Phone Number 1	Phone Number 2	Phone Number 3
333	Choctaw Pass	Auchter, Barbara and Thorne						
126	Sinti trail	Tiller, Micheal D. & Valenc						
140	Sinti Trail	Albritton, Jeffery						
061	Sinti trail	Fellows, Henry & Pamela						
204	Sinti trail	Heller, Steve & Carol						
220	Sinti trail	Morris, William and Margie						
75	Twin Creek Drive	Back, Stephen and Susan						
101	Twin Creek Drive	Psalmond, Phillip and Tami						
115	Twin Creek Drive	Clark, Brandon and Monica				i		
131	Twin Creek Drive	Boudreau, Bill and Cindy						
151	Twin Creek Drive	Creel, Lynda and Micheal						
185	Twin Creek Drive	Santoro, Ann and Peter						
104	Twin Creek Drive	Willett, Roy & Weslie						
80	Twin Creek Drive	Cetrino, Carol/Piccirillo, Mark						

Note:

TABLE $3^{(1)}$ DAM FAILURE NOTIFICATION OF PROPERTIES DOWNSTREAM OF BIG CANOE

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Parcel ID
295	Blackwell Creek Way	049A 064
121	Chastain Court	049 024
59	Clint Court	050 022
916	Cove Lake Drive	049 090 112
976	Cove Lake Drive	049 090 113
968	Cove Lake Drive	049 090 114
903	Cove Lake Drive	049 090 115
141	Cove Lake Drive	049 090 116
779	Cove Lake Drive	049 090 117
661	Cove Lake Drive	049 090 118
543	Cove Lake Drive	049 090 119
112	Cove Lake Drive	049 090 120
459	Cove Lake Drive 049 090 121	
391	Cove Lake Drive 049 090 122	
351	Cove Lake Drive 049 090 123	
303	Cove Lake Drive	049 090 124
249	Cove Lake Drive	049 090 125
8822	Cove Road	049 090
385	Del Moore Road	049 021
11117	Highway 53 East	050 023
10322	Highway 53 East	050 029
100	Hunters Trace	049A 003
1065	Hunters Trace	049A 031
1109	Hunters Trace	049A 032

Note:

⁽¹⁾ Property information provided by the Pickens County EMA on 21 February 2018. Personal names and contact information for residents outside of Big Canoe are not accessible outside of the Pickens County EMA CodeRED system.

TABLE 3⁽¹⁾ DAM FAILURE NOTIFICATION OF PROPERTIES DOWNSTREAM OF BIG CANOE

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

#	Street Name Address	Parcel ID
30	Justice Way	049 030
43	Justice Way	049 031 001
16	Limestone Trail	050 028 001
44	MacKenzie Lane	049 089
533	Old Mill White Road	049 013
1493	Old Mill White Road	049 017
1543	Old Mill White Road	049 020
1985	Old Mill White Road	050 026
2192	Old Mill White Road	050 027
2164	Old Mill White Road	050 027 002
40	Overlook Court	049 090 128
27	Overlook Court	049 090 129
520	Partain Road	049 119
638	Partain Road	049 121
54	Pendley Woods Road 049 124	
24	Pony Lane	049A 017
584	Redbud Pass 049A 051	
451	Redbud Pass 049A 053	
874	Sparkling Springs Road	049 019 001 & 049 019 002
189	Timber Creek	049 089 008
87	Trotters Lane	049A 001
125	Trotters Lane	SEWER PLANT

Note:

⁽¹⁾ Property information provided by the Pickens County EMA on 21 February 2018. Personal names and contact information for residents outside of Big Canoe are not accessible outside of the Pickens County EMA CodeRED system.

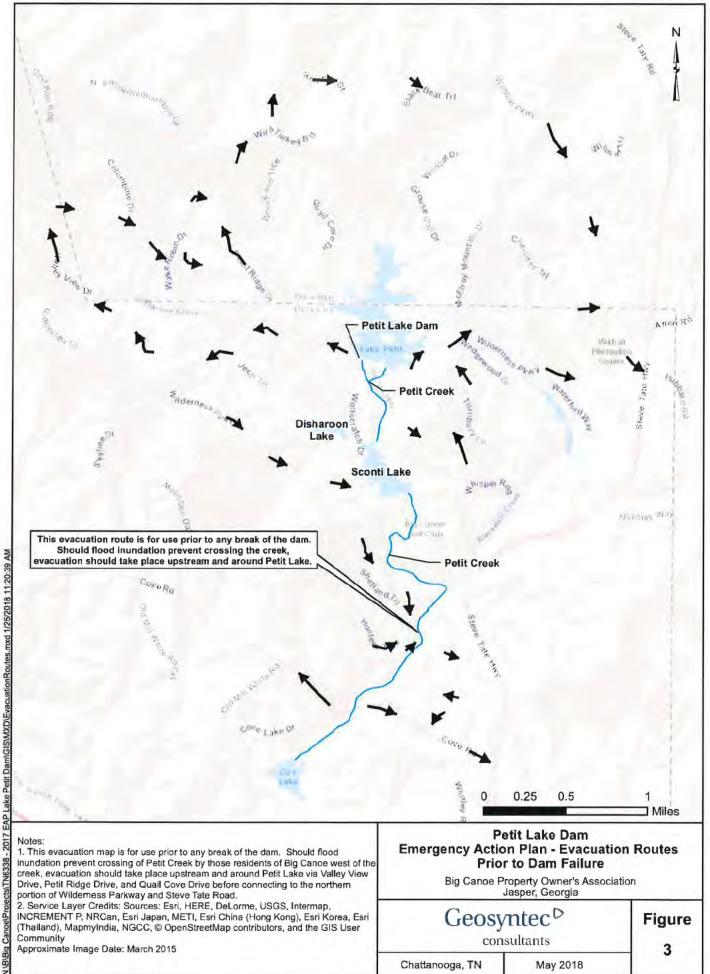
3. PROJECT DESCRIPTION

Petit Lake Dam is located within the Big Canoe Development on Petit Creek about 5.8 miles upstream of Marble Hill, in Pickens County, north central Georgia. The reservoir formed by the dam has a surface area of 104 acres at a normal pool elevation of 1635.0 and extends up Petit Creek approximately 0.70 miles. The total storage for the reservoir is approximately 7,500 ac-ft, according to the Georgia Safe Dams database. The drainage area upstream of Big Canoe Dam is 1.53 square miles. The topography around the dam consists of very steep, wooded, mountainous foothills.

Petit Lake Dam is listed in the Georgia State Safe Dams Program and the National Inventory of Dams under the following identification numbers, respectively: 112-009-00462 and GA00685. The earth dam has a maximum height of 126 feet according to the Georgia Safe Dams database, a length of 880 feet, and a top width of 35 feet. The dam has a 15-foot wide concrete cascading channel spillway on the east side of the earth dam's abutment. The spillway discharge is controlled by a concrete crest underneath a bridge located on the roadway (i.e. Wilderness Parkway) running along the crest of the dam (See Dam Features Map provided in Appendix C). The dam has a 36-inch low level drain which is the only permanent means of lowering the lake level short of structural excavation of an abutment area or the use of temporary pumps or siphons. The low-level drain can be operated by a sluice gate operator located near the center top of the dam. Lake Sconti, owned by Big Canoe, is located some 1.0 miles downstream of Petit Lake. This earth dam has a maximum height 40 feet, a length of 190 feet and a top width of 20 feet. The dam has a 10-foot wide concrete spillway on the eastern abutment. Cox Lake Dam is located some 3.5 miles downstream from Petit Lake Dam south of Cove Road, and is not owned or operated by Big Canoe.

Big Canoe has two permanent roadway access points: (1) the main entrance off Steve Tate Highway is Wilderness Parkway, which crosses Petit Creek some 2.3 miles downstream of Petit Lake Dam, and (2) the secondary entrance off Steve Tate Highway (also Wilderness Parkway) which crosses over the top of Petit Lake Dam, as well as connecting to the main entrance road via a road at the base of Petit Lake Dam (See Inundation Map provided in Appendix B). A third access route utilizes a gravel maintenance road adjacent to the Sconti Golf Clubhouse. These three (3) connections to Steve Tate Highway are the only evacuation routes from the impacted areas of Big Canoe. The use of these evacuation routes will be controlled by the Big Canoe Department of Public Safety to prevent endangerment of evacuees in case of an imminent failure. As shown in Figure 3, two (2) of these evacuation routes would be cut off west of Petit Creek in case of a dam failure of Petit Lake Dam, and evacuation would need to take place via roads upstream of Petit Lake (i.e., Valley View Drive, Petit Ridge Drive, and Quail Cove Drive) and connecting back to the northern intersection of Wilderness Parkway and Steve Tate Road. A small portion of Steve Tate Highway could also be inundated in the event of a major dam failure.

An additional road downstream and outside of the Big Canoe Development would be impacted by failure of Petit Lake Dam. Cove Road is some 3.0 miles downstream of the dam. Petit Creek flows through three (3) 10-foot x 10-foot box culverts under Cove Road.



4. EMERGENCY DETECTION, EVALUATION, AND CLASSIFICATION

Detection and Evaluation of Petit Lake Dam Failure:

Once the observer notifies the Dam Owner's Representative or his designee about a condition of concern, the Dam Owner's Representative or his designee will immediately proceed to Petit Lake Dam and personally inspect the dam for any signs of failure condition. The Dam Owner's Representative or his designee will evaluate the severity of the condition of concern utilizing the guidelines in Table 4 to determine the appropriate Emergency Action Classification. Petit Lake Dam Failure Emergencies will be classified according to their severity and urgency. For the purposes of this EAP, three emergency classifications are provided. Conditions C, B, and A are consistent with other Big Canoe action plans, while Levels 1, 2, and 3 are consistent with Georgia Department of Natural Resources (DNR) Safe Dams Program. For clarity, both nomenclatures are provided throughout this EAP and are defined as follows:

Condition C (Level 1): Failure is imminent or has occurred

This is a situation where a failure either has occurred, is occurring, or obviously is just about to occur. It is impossible to determine how long it will take for a failure to occur or for a complete breach to occur once failure begins. Therefore, once the Dam Owner's Representative determines that there is no longer time available to attempt corrective measures to prevent failure, the "failure is imminent or has occurred" warning, as suggested in Section 5.1, should be issued. Emergency management officials will interpret the phrase "failure is imminent" to mean that the dam is failing. For evacuation purposes, "failure is imminent" and "failure has occurred" should be interpreted as the same condition.

Condition B (Level 2): Potential failure situation is developing

This is a situation where a failure may eventually occur, but preplanned actions taken during certain events (such as major floods, earthquakes, evidence of piping, etc.) may alleviate failure. Recommended methods and steps to alleviate the risk of failure are provided in Table 5. Even if failure is inevitable, more time is generally available than in a "failure has occurred" situation to issue warnings and/or take preparedness actions. When a dam safety situation is observed that may lead to a failure if left unattended, but there is no immediate danger, the Dam Owner's Representative will issue a warning that a "potential failure situation is developing". The Dam Owner's Representative will assess the situation and determine the urgency of the emergency situation. Based on the Dam Owner's Representative's assessment, the first series of notifications should be made and it is up to the Pickens County EMA to determine the subsequent course of action to follow.

NOTE: It should be remembered that it may be appropriate to declare a Condition C (Level 1). However, there should be a smooth transition from Condition B (Level 2) to Condition C (Level 1) when using both conditions.

Condition A (Level 3): Unusual, slowly developing event

This is a condition where a situation is developing, but has not yet threatened the operation or the structural integrity of the dam. The situation should be investigated by the Dam Owner's Representative for development of a potential or imminent dam failure situation. The Dam Owner's Representative will assess the situation and determine a path forward approach. Warnings shall not be issued unless the situation develops into a Condition B (Level 2) situation.

To assist the Emergency Management Agency in selecting their appropriate course of action and to provide a proper transition from Condition A (Level 3) to Condition B (Level 2) or Condition B (Level 2) to Condition C (Level 1), the Big Canoe Public Safety Director will clearly communicate the situation to the EMA. For Condition A (Level 3) and Condition B (Level 2) situations, the Big Canoe Public Safety Director will place the first series of notifications on initial alert, and provide periodic updates on the situation as it develops so that the EMA can assess when they should implement their evacuation procedures. For example, the Dam Owner's Representative will issue an initial warning and periodic updates on the lake level as it rises during flooding conditions and eventually overtops the dam. As the lake rises, a "potential failure situation is developing" warning should be issued with periodic updates on how much time is available before overtopping occurs. Once the dam overtops, a "failure is imminent or has occurred" warning should be issued, as suggested in Section 5.1.

Preplanned actions will be initiated once a Condition B (Level 2) Emergency has been declared and the initial notifications have been completed. The preplanned operations that should be undertaken are shown in Table 5.

TABLE 4⁽¹⁾ DETECTION OF DAM FAILURE AND CLASSIFICATION OF EMERGENCY

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

EVENT	DAM FAILURE MECHANISM	EVALUATION OF FAILURE	CLASSIFICATION OF FAILURE MECHANISM ⁽²⁾
Unexpected Failure	Unknown	Dam unexpectedly and without warning begins to fail	Condition C (Level 1)
Major Flood/		Erosion and removal of the road and embankment occurring	Condition C (Level 1)
Embankment Overtopping	 Overtopping of dam 	Flood pool rapidly approaching top of dam and embankment still intact	Condition B (Level 2)
Overtopping		Settlement of more than a few inches	Condition B (Bever 2)
		Slope movement larger than the size of a car	Condition C (Level 1)
		Flowing water from downstream face of dam	condition c (Bever 1)
Global	 Settlement of dam crest 	Settlement of less than a few inches	
Earthquake or	Slope movement	Slope movement of less that the size of a car	Condition D (Loud 2)
Seismic Activity	Evidence of seepage or piping	Wet areas on downstream face of dam that continue to increase in size and intensity of flow	Condition B (Level 2)
		Measurable earthquake felt or reported near the dam and dam appears to be stable.	Condition A (Level 3)
		Settlement of more than a few inches	Condition C (Level 1)
Embankment	Settlement of dam crest	Slope movement larger than the size of a car	Condition C (Bover 1)
Movement	Slope movement	Settlement of less than a few inches	Condition B (Level 2)
		Slope movement of less than the size of a car	
		New cracks in the embankment greater than 1/4-inch wide without scepage	Condition A (Level 3)
Embankment	Total and the second se	Flowing water greater than 10 gallons per minute from downstream face of dam	Condition C (Level 1)
Seepage	Evidence of seepage or piping	Wet areas with cloudy discharge on downstream face of dam that continue to increase in size and intensity of flow	Condition B (Level 2)
		New seepage areas in or near the dam, water flowing clear	Condition A (Level 3)
	 Spillway overflow 	Spillway overflowing with an advancing head cut that is threatening the control section or that is already flooding people downstream	Condition C (Level 1)
Spillway Flow	Spillway erosion	Spillway overflowing with active gully erosion	Condition B (Level 2)
1		Spillway overflowing with no active erosion	Condition A (Level 3)
		Normal flow with erosion under, beneath, or at edges of the spillway	Condition A (Level 3)
		Rapidly enlarging sinkhole on dam or appurtenances	Condition C (Level 1)
Sinkholes	 Observed sinkhole 	Observation of new sinkhole in reservoir area or on embankment	Condition B (Level 2)
		Observation of sinkhole downgradient of the dam	Condition A (Level 3)
		Increase in piezometer readings of more than 10 feet and flowing water from downstream face of dam	Condition C (Level 1)
		Rapid decrease in lake level and flowing water from downstream face of dam	Continuo (Corror 1)
Routine Instrumentation Readings	 Significant change in piezometer readings Rapid decrease in lake level 	Increase in piezometer readings of more than 10 feet and no flowing water from downstream face of dam	Condition B (Level 2)
		Rapid decrease in lake level with no apparent reason and no flowing water from downstream face of dam	Condition B (Level 2)
		Piezometer readings vary beyond predetermined values and no flowing water from downstream face of dam	Condition A (Level 3)
		Detonated bomb that has results in damage to the dam or appurtenances	Condition C (Level 1)
Security Threat	Bomb threat	Verified bomb threat that, if carried out, could result in damage to the dam or appurtenances with no impacts to the functioning of the dam	Condition B (Level 2)
		Reported bomb threat, unverified	Condition C (Level 1)
		Damage to dam or appurtenances that has resulted in uncontrolled water release	Condition A (Level 3)
Sabotage/ Vandalism	 Damage to dam or appurtenances 	Damage to dam or appurtenances that has resulted in seepage flow	Condition B (Level 2)
y attocitati	аррилсивнеев	Damage or modification to the dam or appurtenances with no impacts to the function of the dam	Condition C (Level 1)
Blocked culverts	Blockage	Debris is blocking a spillway pipe, causing lake level to rise	Condition C (Level 1)

Notes:

(1) This table was based on a similar table from the North Carolina Department of Environmental Quality, 2016.

⁽²⁾ Conditions C, B, and A are consistent with other Big Canoe action plans while Levels 1, 2, and 3 are consistent with Georgia Department of Natural Resources (GA DNR) Safe Dams Program. For clarity, both nomenclatures are provided.

TABLE 5 PREPLANNED OPERATIONS FOR CONDITION B (LEVEL 2) EMERGENCY

EMERGENCY ACTION PLAN FOR PETIT LAKE DAM PETIT LAKE DAM, BIG CANOE, GEORGIA

	IMPENDING DAM	
EVENT	IMPENDING DAM FAILURE MECHANISM	PREPLANNED OPERATIONS IN PRIORITY ORDER
		Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.
Major Flood/Embankment Overtopping	Overtopping of dam	 Open the bypass valve on the water supply line to the water treatment plant (if available). If lake levels continue to rise dangerously close to top of the dam crest, excavate emergency channel in abutment area adjacent to concrete chute spillway. Also bring in emergency pumps and discharge outflow to spillway or directly into Petit Creek.
		Do not excavate channel on top of dam, or discharge pump outflow on face of dam
		Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake.
Earthquake or Seismic Activity	Slope Failure	 Open the bypass valve on the water supply line to the water treatment plant (if available). Monitor piezometer readings daily and plot readings to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings. Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).
Embankment Movement, Embankment Seepage, Spillway Flow, or Sinkhole	Slope Failure	 Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water treatment plant (if available). Monitor piezometer readings daily and plot reading to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings. Obtain input from Georgia Safe Dams Program and Owner's Engineer as to emergency repairs to be constructed (if any).
Routine Instrumentation	Slope Failure	 Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water. Monitor piezometer readings daily and plot reading to identify significant changes in readings. Also record lake levels. Survey elevation along top of dam daily at 50' intervals. Plot elevations of each point to identify significant changes in readings. Obtain input from Georgia Safe Dams program or Owner's Engineer as to emergency repairs to be constructed (if any).
Security Threat or Sabotage	Slope Failure	Make a reasonable attempt to open the sluice gate on the low level drain, or bring in pumps and discharge outflow to spillway or directly into Petit Creek, to lower level of lake. Open the bypass valve on the water supply line to the water treatment plant (if available).

5. GENERAL RESPONSIBILITIES UNDER THE PLAN

The EAP contains preplanned actions designed to prevent a failure to dam structures or to minimize the impact of a dam failure on life and property. It establishes and documents procedures for notifying state and local emergency management agencies, law enforcement bodies, and downstream residents affected by a dam failure. All responsible parties responsible for implementing the EAP shall verify their responsibilities with their signature in Appendix D.

5.1 Dam Owner's Responsibilities:

The person responsible for performing the tasks required under the EAP is the Dam Owner's Representative and the Public Safety Director. If the Public Safety Director is absent, the responsible person will be the Public Safety Director Lieutenant on duty at that time.

The duties of the Public Safety Director under the EAP should be as follows:

- 1. The responsibility for making the initial assessment of the emergency Condition C (Level 1), Condition B (Level 2), or Condition A (Level 3).
- 2. The responsibility for initiating the notification process (see Notification Flowchart). All warning messages should be brief and to the point. For example, for a Condition C (Level 1) Emergency, the message should be as follows: "This is Public Safety Director at Big Canoe. I have verified overtopping of Petit Lake dam and believe dam failure is imminent. I request that you immediately activate the Petit Lake Dam Emergency Action Plan. This is an actual emergency and not a drill." Note that a warning message shall not be issued for a Condition A (Level 1) emergency.
- 3. The responsibility for initiating the preplanned actions in case of a Condition B (Level 2) emergency.

5.2 Responsibility for Notification:

For a Condition C (Level 1) or Condition B (Level 2) emergency, the Dam Owner's Representative, the General Manager of the Big Canoe POA, is responsible for contacting the Public Safety Director at Big Canoe, the Public Information Officer Communications Manager, and the Liaison Officer, Director of Administrative Services.

The Public Safety Director at Big Canoe is responsible for notifying Pickens County EMA, Critical Infrastructure at Big Canoe (Table 1), Dawson County 911 Services, and the Georgia DNR 24-Hour Emergency Contact in the event of an emergency. If time allows, he should seek advice and assistance.

The Public Information Officer, Communications Manager is responsible for notifying the News Media (including radio and television media). The Liaison Officer, Director of Administrative Services is responsible for notifying Big Canoe's Engineer of Record.

The Pickens County EMA is responsible for notification of the residents below the dam as identified in the Reverse 911 System, including: (i) Big Canoe residents (Table 2) and residents downstream from Big Canoe (Table 3), (ii) Pickens County Sheriff's Department, (iii) Pickens County Public Works, (iv) the Cox Lake Dam Owner (Cove Lake Property Owner's Association, Inc.), and the (v) National Weather Service Weather Forecasting Office.

The Georgia DNR 24 Hour Emergency Contact is responsible for notification of the Georgia Safe Dams Program.

For a Condition A (Level 3) emergency, the Dam Owner's Representative, the General Manager of the Big Canoe POA, is responsible for contacting the Public Safety Director at Big Canoe and the Liaison Officer, Director of Administrative Services.

The Liaison Officer, Director of Administrative Services is responsible for contacting the Big Canoe Engineer of Record. The Public Safety Director at Big Canoe is responsible for contacting the Georgia Safe Dams Program, if required by the Dam Owner's Representative.

5.3 Responsibility for Evacuation:

Warning and evacuation planning are the responsibilities of the Pickens County EMA who have the statutory obligation. Under the EAP, the Big Canoe Public Safety Director is responsible for notifying the Pickens County EMA when a failure is imminent or has occurred (Condition C or Level 1), or a potential failure situation is developing (Condition B or Level 2). Big Canoe will not assume the responsibility of government entities for evacuation of people. This procedure should be coordinated with the appropriate public officials prior to an emergency situation developing.

5.4 Responsibility for Duration, Security Termination, and Follow-Up:

The Pickens County EMA is responsible for monitoring the situation at the dam and keeping local authorities informed of developing conditions at the dam from the time that an emergency starts until the emergency has been terminated. Security measures at the dam should be implemented by the Pickens County Sheriff's Department.

The Pickens County EMA is responsible for declaring that the emergency at the dam is terminated.

A follow-up evaluation after an emergency by all participants will be conducted. The Big Canoe POA is responsible for coordinating this effort and generating a written report which will detail the extent to which the EAP was followed, and what changes, additions or improvements need to be made to the EAP.

5.5 EAP Coordinator Responsibility

The Dam Owner's Representative or his designee will be the designated EAP coordinator who will be responsible for EAP related activities, including preparing revisions to the EAP, establishing training activities, coordinating EAP exercise, etc. They will also be the EAP contact if any of the involved parties have questions about the plan. The EAP will be reviewed annually with contacts, phone numbers, verified for accuracy. Documentation of the annual review is provided in Appendix E. As infrastructure and homes are built in the inundation map hazard areas, contact information (i.e., resident's names, addresses, and phone numbers) will be added to Tables 1, 2, and 3, as appropriate and available.

6. PREPAREDNESS

The following actions should be taken in preparation of an emergency:

- Initiation of a dam inspection and surveillance program. The dam should be formally
 inspected annually, at minimum, and readings from the dam instrumentation should be
 taken and interpreted quarterly, at minimum. During quarterly readings, basic
 observations of the dam can also be made.
- Preparation of a systematic warning and evacuation plan. A formal notification system should be coordinated with residents and businesses in the inundation area, and evacuation routes should be discussed/provided to those residents.
- Preparations should be made for evacuation on weekends, weekdays, and any time of day or night, including holidays. The emergency responders should have backup ways of communicating and a way to respond to the emergency in case of power outages.
- Community awareness programs for emergency response procedures. The community should be made aware of the possible emergencies and procedures associated with the possible failure of the dam.
- Establishment of emergency flood operating procedures.
- Organization of equipment, labor, and materials for use in emergency situations. A
 list of locally available resources that could be used in the event of an emergency are
 provided in Appendix F. These businesses can supply pumps, power generators, divers
 for inspections, and materials for temporary repair, depending on the emergency. Also
 the Pickens County EMA has provided a list of equipment that may be available during
 a dam failure.
- Conduct emergency exercises. At minimum, an orientation meeting should be held
 with key people so that those playing key roles and those having responsibilities
 outlined in this EAP can become familiar with it. A drill or tabletop exercise can be
 held, and coordinated with the Pickens County EMA. Documentation of reviews, tests,
 and revisions of this EAP should be documented in the forms provided in Appendix
 E.

7. REFERENCES

- Association of State Dam Safety Officials [2010]. "EAPWG Final EAP Template," available online at http://www.damsafety.org/community/owners/?p=3a95437d-1876-46d6-843b-d65d45beb46a, accessed March 2017.
- Georgia Safe Dams Program [2015]. "Engineer Guidelines, 2015 Edition," Georgia Department of Natural Resources, Version 4.0.
- Jordan, Jones, and Goulding [1998]. "Emergency Action Plan for Dam Failure, Petit Lake Dam, Big Canoe, Georgia," prepared for Big Canoe Property Owner's Association, September.
- Jordan, Jones, and Goulding [2007]. "Emergency Action Plan for Dam Failure, Petit Lake Dam, Big Canoe, Georgia," prepared for Big Canoe Property Owner's Association, April.
- North Carolina Department of Environmental Quality [2016]. "Guidelines for Owner Completion of the Emergency Action Plan (EAP) Template," Division of Energy, Mineral and Land Resources, available online at https://deq.nc.gov/about/divisions/energy-mineral-land-permits/dam-safety, accessed March 2017.

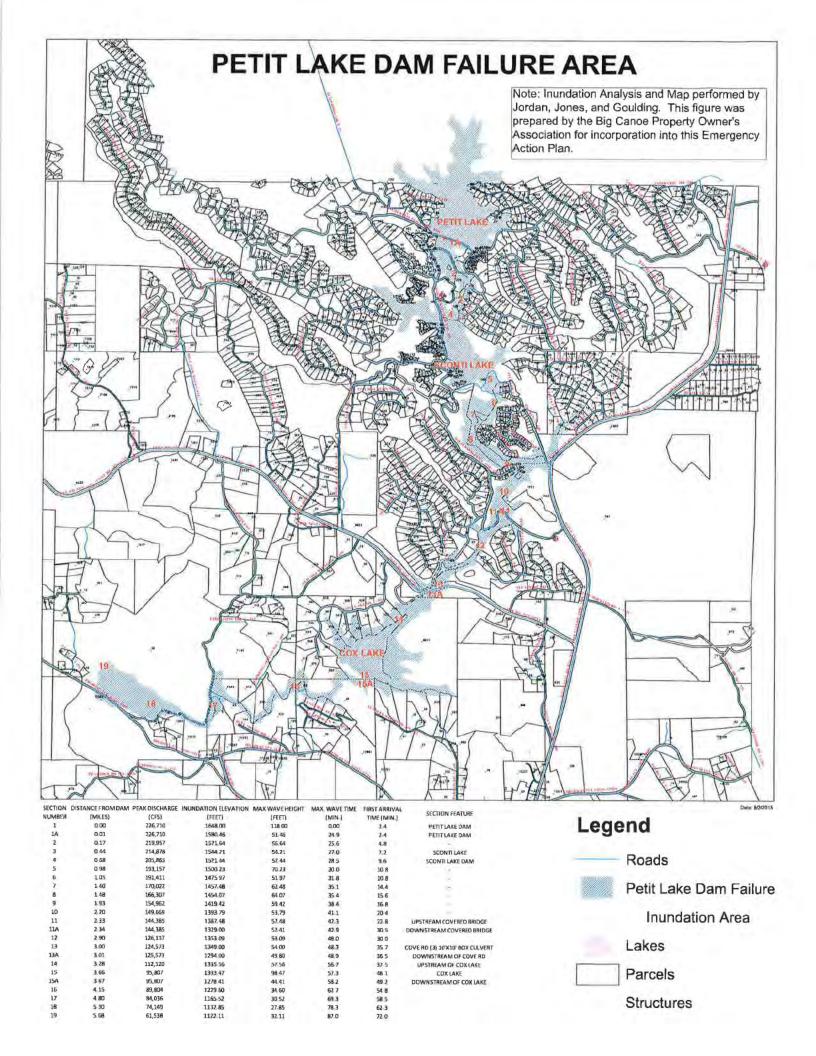
APPENDIX A DEFINITIONS

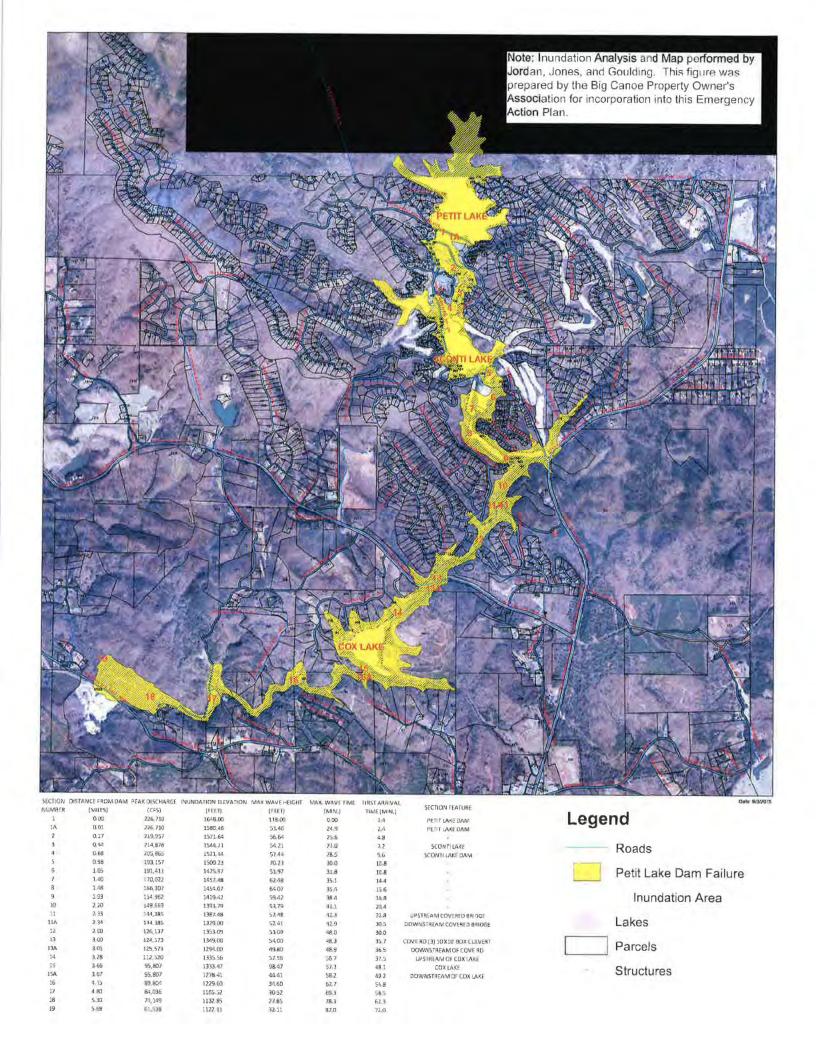
APPENDIX A - DEFINITIONS

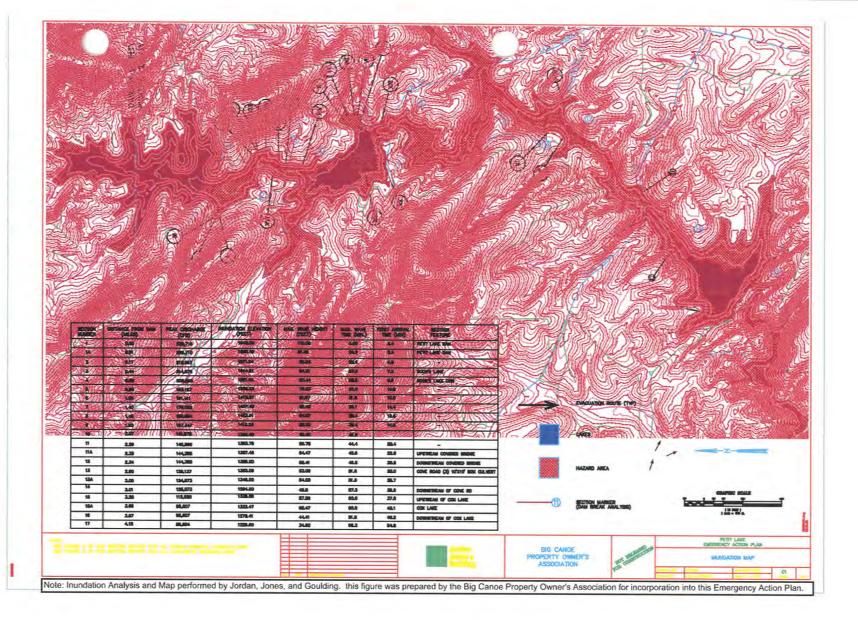
ABUTMENT	The part of the valley side against with the dam is constructed. Right and left abutments are those on respective sides as an observer when looking downstream.
APPURTENANCES	Structures around a dam that are necessary to the operation of the dam project (i.e., spillways).
BREACH	An opening through the dam resulting in partial or total failure of the dam.
CATEGORY I DAM	A dam that is either 25 feet tall or impounds 100 acre-feet of water and where improper operation or dam failure would result in the probable loss of human life.
COMPREHENSIVE EAP EXERCISE	An in-depth exercise of an EAP that involves the interaction of the dam owner with the state and local emergency management agencies in a stressful environment with time constraints. Functional and full scale EAP exercises are considered comprehensive EAP exercises.
DAM FAILURE	Catastrophic type of failure characterized by the sudden, rapid, and uncontrolled release of impounded water. It is recognized that there are lesser degrees of failure and that any malfunction or abnormality outside the design assumptions and parameters which adversely affect a dam's primary function of impounding water is properly considered a failure. Such lesser degrees of failure can progressively lead to or heighten the risk of a catastrophic failure. They are, however, normally amendable to corrective action.
EMBANKMENT	Fill material, usually earth or rock, placed with sloping sides.
EMERGENCY ACTION PLAN (EAP) EXERCISE	An activity designed to promote emergency preparedness; test or evaluate emergency action plans, procedures or facilities; train personnel in emergency management duties; and demonstrate operational capability. Exercises consist to the performance of duties, tasks or operations very similar to the way they would be performed in a real emergency. However, the exercise performance is
EMERGENCY	in response to a simulated event. The State and local agencies responsible for emergency operations, planning,
MANAGEMENT AGENCY	mitigation, preparedness, response, and recovery for all hazards.
EMERGENCY OPERATIONS CENTER	The location or facility where responsible officials gather during an emergency to direct and coordinate emergency operations, to communicate with other jurisdictions and with field emergency forces, and to formulate protective action decisions and recommendations during an emergency.
FLOOD ROUTING	A process of determining progressively over time the amplitude of a flood wave as it moves past a dam or downstream to successive points along a river or stream.
HAZARD	A situation which creates the potential for adverse consequences such as loss of life, property damage or other adverse impacts. Impacts may be for a defined area downstream of a dam from flood-waters released through spillways and outlet works of the dam or waters released by partial or complete failure of the dam.
HEADWATER	The water immediately upstream from a dam. The water surface elevation varies due to fluctuations in inflow and the amount of water passed through the dam.
INUNDATION MAP	A map delineating areas that would be flooded as a result of a dam failure or other unusually large spillway release.
OVERTOP	Flow of an embankment dam beyond its spillway capacity and over the top of the dam crest, or containment elevation.

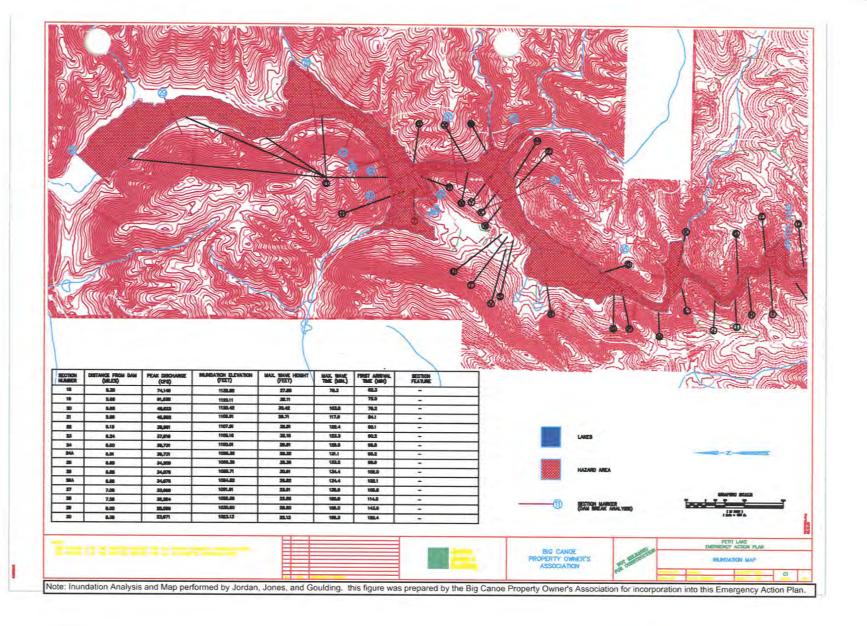
PIPING	Internal erosion caused by seepage.
PROJECT DESIGN FLOOD	The maximum rate of rainfall in which the dam could safely pass or store without overtopping.
SEEPAGE	The continuous movement of water from the upstream face of the dam toward its downstream face.
SETTLEMENT	The downward movement of the ground due to forces (i.e., buildings and other structures) applied to the surface.
SPILLWAY	A structure over or through which flood flows are discharged.
TAIL WATER	The water downstream from the dam.

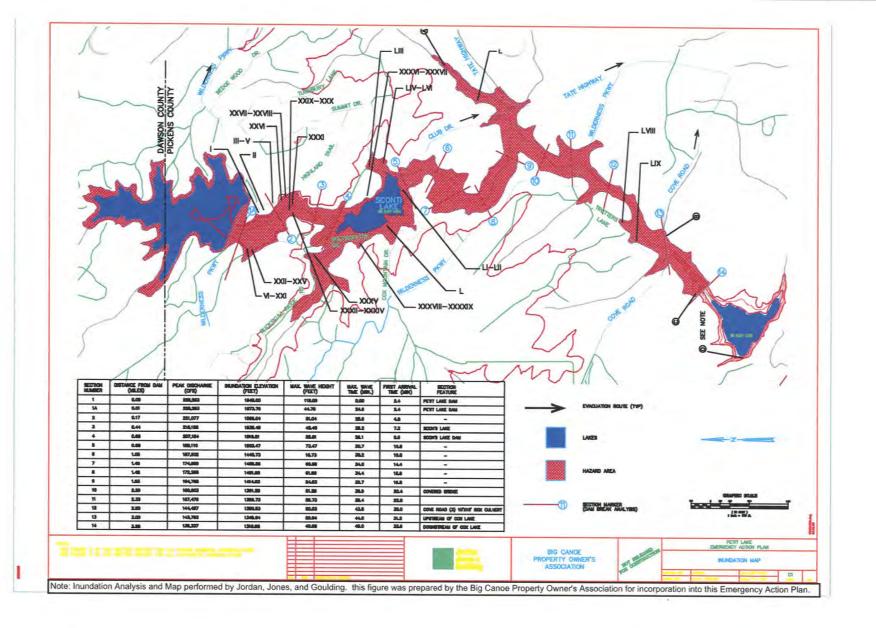
APPENDIX B INUNDATION MAPS AND CALCULATIONS PREPARED BY JORDAN, JONES, AND GOULDING AND BIG CANOE POA

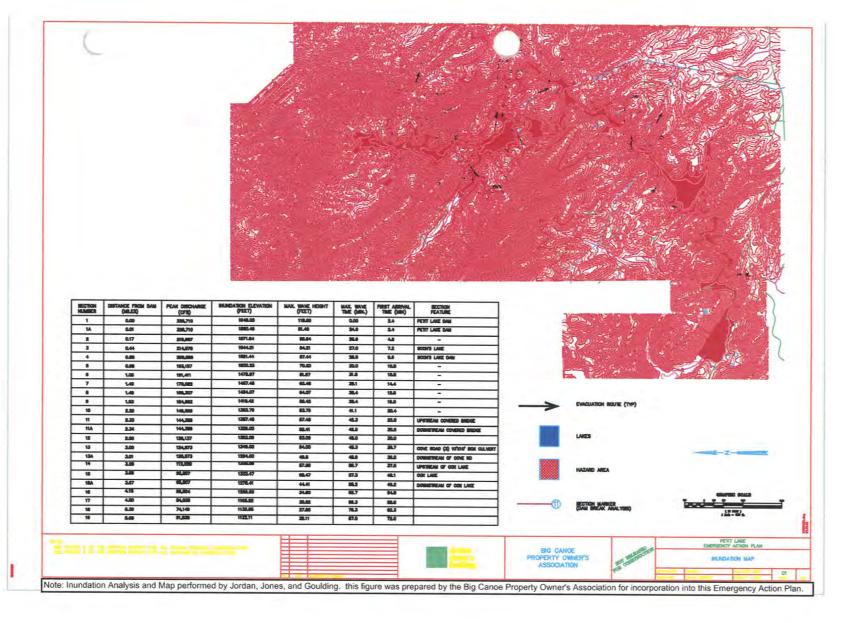


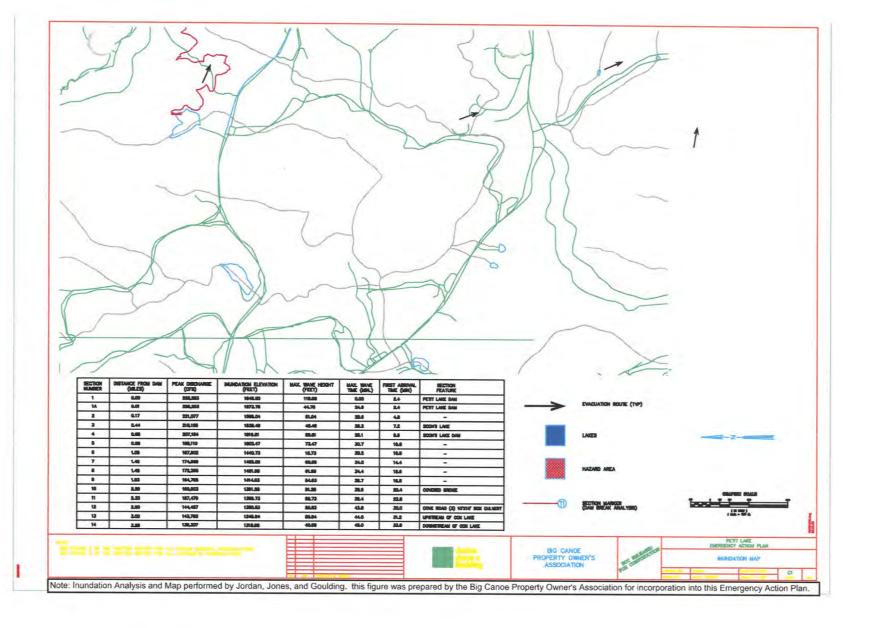












APPENDIX B Page 1

Area Description

Petit Lake is a 115 foot high earthen embankment in the upper reaches of the Blackwell Creek drainage basin in northeastern Pickens County. It impounds approximately 6,000 acre-feet of water, according to National Inventory of Dams data (1992). The contributing drainage area is approximately 1.53 square miles consisting primarily of heavily wooded residential development and heavily wooded undeveloped land.

Approximately 1.0 mile downstream of Petit Lake on Blackwell Creek is Sconti Lake. The embankment here is approximately 40 feet high and only impounds approximately 260 acre-feet according to the National Inventory of Dams data (1992). The drainage area to Sconti, minus the area into Petit, is approximately 3.3 square miles, again consisting primarily of heavily wooded residential and heavily wooded undeveloped land.

Between the two lakes, the valley is partially cleared and serves primarily as a golf course and for other recreational activities. Downstream of Sconti the valley is relatively narrow, typical of North Georgia undeveloped streams, and is generally heavily wooded.

Approximately 1.33 miles downstream of Sconti is a two-lane roadway (Wilderness Parkway) crossing of Blackwell Creek consisting of a covered bridge approximately 100 feet in length. The roadway connects to the south main entrance to the Big Canoe community.

Approximately 0.6 miles further downstream is another two-lane roadway crossing named Cove Road. This is a county maintained road. This crossing consists of three 10' x 10' box culverts.

Downstream of Cove Road, according to the U.S.G.S. Nelson Quadrangle Map, is Cox Lake. The embankment is approximately 97 feet high

Downstream of Cox Lake, the stream is again relatively narrow and heavily wooded. There are several homes at various points along the stream in this area and the stream adds some additional tributaries. Just east of the Marblehill community and parallel to the north of Georgia Hwy. 53, the stream widens into a small floodplain consisting of cultivated fields.

From here, the stream curls to the northwest and then south around a small rise and goes under Hwy. 53 in Marblehill. It then turns to the northeast to travel through Marblehill, going under a railroad bridge in the center of Marblehill.

From Marblehill the stream enters a relatively wide floodplain extending to the east of Marblehill and parallel to the south of Hwy. 53.

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Modeling

The stream from Petit Lake to the floodplain east of Marblehill has been modeled using the National Weather Service program, DAMBRK. The dam breach flood volume and peak downstream times, elevations and flows have been calculated using this program.

Cox Lake has been modeled using design plans provided by the NCRS office in Jasper, Georgia. It was initially hoped that the flood wave would be stopped and held in this structure. However, the magnitude of the flood wave is such that the dam is overtopped with substantial flow occurring through the emergency spillway and over the top of the dam. The height of the modeled overtopping is approximately 1.25 feet or 0.75 feet below the breach elevation (two feet above top of dam). It is assumed since this is an engineered structure and has been periodically inspected through the years, it would be able to withstand this overtopping without breaching.

The covered bridge road and Cove Road were both modeled as dams with minimal initial flow-through for initial conditions computations. The area of opening capacity for both roads below the roadway elevation is insignificant compared to the magnitude of the breach flood peak, therefore it is not necessary to explicitly model the bridge openings.

In contrast, the Hwy. 53 and railroad bridge in Marblehill were modeled as bridges with the area of opening below the embankment defined. The two crossings were field measured and both are in good structural condition. However, the breach wave height exceeds the embankment height by more than two feet for both crossings resulting in breaching of both crossings.

Worst Case Scenario

Two scenarios are generally appropriate for modeling a dam breach wave. One scenario would consist of a sunny day breach which have the model beginning with a full reservoir to the top of dam elevation with all outflow structures blocked. The other would consist of a project storm breach consisting of water level beginning at normal pool, normal outflow occurring, and a project storm hydrograph entering the reservoir with the breach occurring at the peak time of the inflow hydrograph. The two scenarios would be compared and the worst case used to define the dam breach flood zone. For dams with large storage capacities and large drainage areas, the two scenarios are generally necessary for determining the worst case for downstream flooding.

Petit Lake has a large storage capacity and small inflow drainage basin. Only one breach scenario has been modeled for this EAP, the sunny day breach. The storm breach analysis was limited because the excess storage capacity before the dam is overtopped minus the spillway outflow is adequate to contain the storm runoff from this relatively small sparsely developed drainage basin. The peak breach wave from this model at Petit Lake is 225,253 cubic feet per second. With the starting water surface level at the top of dam elevation, the freeboard is completely filled.

Preliminary project storm hydrographs have been developed for both Petit Lake and Sconti Lake using HEC-1. The hydrographs are for PMP rainfall, taken from Hydrometeorological Report No. 51 from the National Weather Service, consisting of 30.2 and 41 inches for the 6-Hour and 24-Hour duration storms, respectively. A Curve Number and Lag Time have been determined using Soil Conservation Service (now National Resource Conservation Service) techniques. The peak flow for Petit Lake for the two storms is 14,624 cfs (6-Hour) and 7,100 cfs (24-Hour). This is less than 10% of the modeled initial peak breach flow. For Sconti Lake (not including the Petit Lake drainage area) the two peaks are 18,830 cfs (6-Hour) and 13,294 cfs (24-Hour), again less than 10% of the breach flow.

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During the project storm scenario, the freeboard storage and outflow capacity of Petit Lake would cause the dam breach to begin at a level substantially below the top of dam elevation, thus producing a lowered volume and peak when compared to the sunny day scenario. The project storm hydrograph into Sconti Lake would not raise a total project storm breach flow peak to above the level of the sunny day scenario.

Because of the large storage capacity of Petit Lake and the relatively small PMP inflows to both Petit and Sconti Lakes, we are confident the sunny day scenario represents the worst case. The mapped extents of the sunny day model represents the maximum area of flood that would occur for any breach scenario.

Modeling Parameters

Modeling parameters have been developed in accordance with the Georgia Safe Dams Program Engineering Guidelines, 1998 Edition (1). The following breach parameters have been used:

Petit Lake

Starting Water Surface Elevation = Top of Dam (1648 msl)

(1) Breach Base Width = Height of Dam (118 feet)

Breach Side Slope = 1:1

(1) Time to Complete Failure = 0.5 hours

Sconti Lake

Starting Water Surface Elevation = Normal Pool (1464.3 msl, approx.)

(1) Breach Base Width = Height of Dam (40 feet)

Breach Side Slope = 1:1

(1) Time to Complete Failure = 0.5 hours

Covered Bridge

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (31 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Cove Road

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (30 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Cox Lake

Starting Water Surface Elevation = Normal Pool (1276.2 msl)

(1) Breach Base Width = Height of Dam (97 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.5 hours

Hwy. 53 in Marblehill

Starting Water Surface Elevation - Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (22 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

Railroad Bridge in Marblehill

Starting Water Surface Elevation = Calibrated for Initial Conditions

(1) Breach Base Width = Height of Embankment (22 feet)

Breach Side Slope = 1:1

Time to Complete Failure = 0.1 hours

APPENDIX B Page 4

Manning's values and expansion/contraction coefficients were based on engineering judgment after visual inspection of the Blackwell Creek valley. Sections were obtained from a digital terrain model developed from U.S.G.S. quad topography and were adjusted to reflect visual inspection of the stream valley channel. Minor adjustments were made in stream reach distances to promote numerical stability in the model.

Areas of off-channel storage have been incorporated at appropriate locations in the model.

Additional parameters were included or modified based on requirements for calculating initial conditions and for promoting numerical stability through a complete peak assessment for all sections. These additional parameters include: section smoothing, delta x distances, constant turbine flow, additional reservoir inflow, lateral inflow, and others as required. These additional parameters serve only to promote the numerical stability of the model and do not minimize the magnitude of the peak elevation and flow results.

Results

The modeling results indicate that Sconti Lake Dam, the covered bridge road, and Cove Road would all be breached under a substantial flood wave.

Peak elevations indicate that a small portion of Steve Tate Road would be underwater.

The model indicates that Cox Lake would be overtopped but not breached. Portions of Hwy. 53 to the east and to the west of Marblehill would be temporarily submerged but are on the edge of the main flood channel. The crossings in Marblehill would be breached and areas in Marblehill within the floodplain south of Hwy. 53 would be inundated and in the direct path of the breach wave.

The model indicates that the breach wave is dissipated to a height of approximately 22 feet just upstream of where Hwy. 53 again crosses the stream, east of Marblehill. The height of the bridge/embankment here is approximately 27 feet. It is therefore assumed this crossing serves to break the flood wave and dissipate the volume in the upstream floodplain.

The extent of the breach flooding is shown on the attached maps. DAMBRK and HEC-1 input and output is attached.

148.8 1650.0 0.0 1648.	1 103.9 1635.0 1648.	1 6 66.5 1620.0 1.0 0.0	4 13 34.7 1600.0 1530.	3 16 14.6 1580.0 118.	3 4.2 1560.0 0.5 0.0	1.1 1540.0 1530.	0.0 1530.0 0.0 1100.0	
0.0 1472.0 0.0 1349. 0.0 1327 0.0 100.0	1464.30 1470. 1335.0 1347. 1300.0 1325. 0.81 1300.0 2.0	1.0 1464.0 1.0 1330.0 1.0 1295.0 1.0 10000.0 24.0	1430.0 0.0 1330.0 0.0 1295.0 0.0	40 318.0 31.0 45.0 30.0 45.0	0.5 0.0 0.1 0.0 0.1 0.0	1430.0 570.0 1330.0 450.0 1295.0 750.0	0.0 975.0 0.0 0.0 0.0 0.0	
	18 1 0.9 2 3 5 7 8 9	8 2 5 3 4 6 8 9 10	6 6 7	0 13	-1 16	2 18	2	0
0.0 1530. 300. 0.	1540. 450.	1560. 630.	1580. 840.	1600. 1070.	1620. 1600.	1640. 1940.	1650. 2300.	
0.01 1529.0 300. 0.	1540. 450.	1560. 630.	1580. 840.	1600. 1070.	1620. 1600.	1640. 1940.	1650. 2300.	
1515. 40. 0. 0.44	1520. 105.	1540. 300.	1560. 680.	1580. 770.	1590. 850.	1600. 980.	1610. 1190.	
1490. 13. 0. 0.68	1495. 35.	1500. 165.	1510. 250.	1520. 315.	1540. 432.	1560. 560.	1564 - 620 -	
1464. 5. 0. 0.98	1470. 10.	1480. 180.	1490. 470.	1500. 1060.	1510. 1180.	1520. 1230.	1540. 1450.	
1430. 5. 0. 0.99	1460. 100.	1470. 276.	1480. 356.	1490. 506.	1500. 680.	1505. 960.	1540. 2395.	
1429.0 5. 0. 1.05	100.		356.	506.	680.	1505. 960.	1540. 2395.	
1424. 19. 0. 1.40	39.	1430. 180.	263.	350.	637.	785.	1484 - 978 .	
1395. 29. 0. 1.48	65.		420.	510.	1430. 590.	695.	1488. 1185.	
1390. 21. 0. 1.93	1395. 30.	1400. 55.	1405. 170.	1420. 290.	1430. 380.	1440. 455.	1462. 590.	
1360. 18. 0. 2.20	1365. 41.	125.		1400. 320.	1420. 435.	1440. 560.	1450. 650.	
1340. 28. 0. 2.33	1345. 46.	1350. 140.	1360. 240.	1380. 425.	1390. 520.	1420. 710.	1430. 790.	
	1340. 90.	1350. 330.	1360. 470.	1380. 700.	1390. 790.	1400. 845.	1410. 1020.	

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's

0.							
2.34							
1329	0 1340-	1350.	1360.	1380.	1390.	1400.	1410.
17.	90.	330.	470.	700.	790.	845.	1020.
0.				,	25.55		10201
2.90							
1300.	1305	1310.	1320.	1330.	1340.	1360.	1390.
23.	40.	145.	285.	380.	450.	630.	850.
0.		143.	205.	500.	450.	030.	030.
3.00							
1295.	1325.	1330.	1340.	1350.	1360.	1370.	1380.
30.	250.	540.	830.	985.	1135.	1290.	
0.	230.	240.	230.	703.	(133.	1290.	1450.
3.01							
1294.	0 1325.	1330.	17/0	1750	17/0	4770	1700
30.	250.		1340.	1350.	1360.	1370.	1380.
	230.	540.	830.	985.	1135.	1290.	1450.
0.							
3.28	4200	1700	4700	4710	47/0	4700	
1278.		1300.	1320.	1340.	1360.	1380.	1400.
35.	115.	300.	530.	665.	780.	915.	1095.
0.	1.11	2.12	2 327	2.72	0.04	-1.22	
0.050		0.10	0.12	0.15	0.15	0.18	0.18
0.050		0.10	0.12	0.15	0.15	0.18	0.18
0.050		0.10	0.12	0.15	0.15	0.18	0.18
0.050		0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.060	0.07	0.10	0.12	0.15	0.15	0.18	0.18
0.060	0.07	0.10	0.12	0.15	0.15	0.18	0.18
0.050	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.050	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.050	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.050	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0,055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.055	0.06	0.10	0.12	0.15	0.15	0.18	0.18
0.05	0.00	0.05	0.05	0.05	0.00	0.05	0.00
0.05	0.05	0.05	0.00	0.00	0.00	0.05	0.00
0.00	0.00			0.00	0.00	0.03	0.00
0.0	0.00	0.25	0.0	-0.4	0.00	0.00	0.25
-0.4	0.25	0.00	0.00	0.00	0.00	0.00	-0.3
0.00	0.23	0.00	0.00	0.00	0.00	0.00	-0.5
0.0	0.0	-27.0	0.0	0.0	0.0	0.00	0.0
2	7	-21.0	0.0	0.0	0.0	0.00	0.0
50.0	500.0	1000.0					
00.0	000.0						
00.0	000.0	1000.0					

ANALYSIS OF THE DOWNSTREAM FLOOD HYDROGRAPH PRODUCED BY THE DAM BREAK OF

Emerg. Action Plan

ON

Lake Petit Dam

ANALYSIS BY

Jordan, Jones, & Goulding

BASED ON PROCEDURE DEVELOPED BY DANNY L. FREAD, PH.D., SR. RESEARCH HYDROLOGIST

QUALITY CONTROL TESTING AND OTHER SUPPORT BY JANICE M. LEWIS, RESEARCH HYDROLOGIST

HYDROLOGIC RESEARCH LABORATORY W23, OFFICE OF HYDROLOGY NOAA, NATIONAL WEATHER SERVICE SILVER SPRING, MARYLAND 20910

****	******	***	*****	****	****
****	****	***	****	****	***
***					***
***	SUMMARY	OF	INPUT	DATA	***
***					***
****	*****	***	*****	*****	***
****	****	***	*****	****	****

INPUT CONTROL PARAMETERS FOR Emerg. Action Plan

PARAMETER ***********************************	VARIABLE	VALUE ******
NUMBER OF DYNAMIC ROUTING REACHES	KKN	1
TYPE OF RESERVOIR ROUTING	KUI	1
MULTIPLE DAM INDICATOR	MULDAM	4
PRINTING INSTRUCTIONS FOR INPUT SUMMARY	KDMP	3
NO. OF RESERVOIR INFLOW HYDROGRAPH POINTS	ETEH	3
INTERVAL OF CROSS-SECTION INFO PRINTED OUT WHEN JAN	C=9 NPRT	0
FLOOD-PLAIN MODEL PARAMETER	KFLP	0
METRIC INPUT/OUTPUT OPTION	METRIC	0

IDAM= 1

IDAM= 6

IDAM= 13

IDAM= 16

Emerg. Action Plan RESERVOIR

TABLE OF ELEVATION VS SURFACE AREA

SURFACE AREA (ACRES) ELEVATION (FT)
SA(K) HSA(K)

148.8 1650.00 103.9 1635.00

34.1	1600.00
14.6	1580.00
4.2	-1560.00
1.1	1540.00
.0	1530.00

DAM NUMBER 1

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER ********************************	UNITS V	ARIABLE	VALUE *******
ELEVATION OF WATER SURFACE	FEET	YO	1648.00
SIDE SLOPE OF BREACH		z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1530.00
WIDTH OF BASE OF BREACH	FEET	ВВ	118.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.50
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1648.00
ELEVATION OF TOP OF DAM	FEET	HD	1648.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		CS	.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	.00
DISCHARGE THRU TURBINES	CFS	QT	1100.00

CDO SHOULD NOT BE 0.0 IF OVERTOPPING MAY OCCUR

DAM NUMBER 2

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER	UNITS Y	ARIABLE	VALUE	
*******************	****	*****	*****	
ELEVATION OF WATER SURFACE	FEET	YO	1464.30	
SIDE SLOPE OF BREACH		Z	1.00	
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1430.00	
WIDTH OF BASE OF BREACH	FEET	BB	40.00	
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.50	
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1472.00	
ELEVATION OF TOP OF DAM	FEET	HD	1470.00	
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1464.00	
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00	
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		cs	318.00	

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's

DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	570.00
DISCHARGE THRU TURBINES	CFS	QT	975.00

DAM NUMBER 3

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER **************************	UNITS	VARIABLE	VALUE
ELEVATION OF WATER SURFACE	FEET	YO	1335.00
SIDE SLOPE OF BREACH		z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1330.00
WIDTH OF BASE OF BREACH	FEET	ВВ	31.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.10
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1349.00
ELEVATION OF TOP OF DAM	FEET	HD	1347.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1330.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		CS	45.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	450.00
DISCHARGE THRU TURBINES	CFS	QT	.00

DAM NUMBER 4

Emerg. Action Plan RESERVOIR AND BREACH PARAMETERS

PARAMETER	UNITS V	ARIABLE	VALUE
**********	*****	*****	*****
ELEVATION OF WATER SURFACE	FEET	YO	1300.00
SIDE SLOPE OF BREACH		Z	1.00
ELEVATION OF BOTTOM OF BREACH	FEET	YBMIN	1295.00
WIDTH OF BASE OF BREACH	FEET	ВВ	30.00
TIME TO MAXIMUM BREACH SIZE	HR	TFH	.10
ELEVATION OF WATER WHEN BREACHED	FEET	HF	1327.00
ELEVATION OF TOP OF DAM	FEET	HD	1325.00
ELEVATION OF UNCONTROLLED SPILLWAY CREST	FEET	HSP	1295.00
ELEVATION OF CENTER OF GATE OPENINGS	FEET	HGT	.00
DISCHARGE COEF. FOR UNCONTROLLED SPILLWAY		ĊS	45.00
DISCHARGE COEF. FOR GATE FLOW		CG	.00
DISCHARGE COEF. FOR UNCONTROLLED WEIR FLOW		CDO	750.00
TAX	the same of the same of the same of		

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's

DISCHARGE THRU TURBINES

TO 27

- 00

DHF(INTERVAL BETWEEN INPUT HYDROGRAPH ORDINATES) = .00 HRS.

TEH(TIME AT WHICH COMPUTATIONS TERMINATE)= .8100 HRS.

BREX(BREACH EXPONENT) = 1.000

MUD (MUD FLOW OPTION) = 0

IWF(TYPE OF WAVE FRONT TRACKING) = 0

KPRES(WETTED PERIMETER OPTION) = 0

KSL(LANDSLIDE PARAMETER) = 0

DFR(WINDOW FOR CRITICAL FROUDE NO. IN MIX FLOW ALGORITHM)=

- 050

INFLOW HYDROGRAPH TO Emerg. Action Plan

100.00 1300.00 10000.00

TIME OF INFLOW HYDROGRAPH ORDINATES

.0000 2.0000 24.0000

CROSS-SECTIONAL PARAMETERS FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER ***********************************	VARIABLE *****	VALUE *****	
NUMBER OF CROSS-SECTIONS	NS	18	
MAXIMUM NUMBER OF TOP WIDTHS	NCS	8	
NUMBER OF CROSS-SECTIONAL HYDROGRAPHS TO PLOT	NTT	6	
TYPE OF OUTPUT OTHER THAN HYDROGRAPH PLOTS	JNK	0	
CROSS-SECTIONAL SMOOTHING PARAMETER	KSA	-1	
DOWNSTREAM SUPERCRITICAL OR NOT	KSUPC	2	
NO. OF LATERAL INFLOW HYDROGRAPHS	LQ	2	
NO. OF POINTS IN GATE CONTROL CURVE	KCG	0	

1 2 6 13 16 18

SMF= .90 NTSM= 5 NSMR= 7

NUSM(K)	NDSM(K)
2	3
3	4
5	6
7	8
8	9
9	10
10	11

CROSS-SECTIONAL VARIABLES FOR take Petit Dam BELOW Emerg. Action Plan

PARAMETER	UNITS	VARIABLE
*********	*****	****
LOCATION OF CROSS-SECTION	MILE	XS(1)
ELEVATION(MSL) OF FLOODING AT CROSS-SECTION		FSTG(I)
ELEV CORRESPONDING TO EACH TOP WIDTH	FEET	HS(K,I)
TOP WIDTH CORRESPONDING TO EACH ELEV (ACTIVE FLOW PORTION)	FEET	BS(K,I)
TOP WIDTH CORRESPONDING TO EACH ELEV (OFF-CHANNEL PORTION)	FEET	BSS(K,I)
NUMBER OF CROSS-SECTION		1
NUMBER OF ELEVATION LEVEL		K

XS(1) = .000 FSTG(1) = .00

HS ... 1530.0 1540.0 1560.0 1580.0 1600.0 1620.0 1640.0 1650.0 BS ... 300.0 450.0 630.0 840.0 1070.0 1600.0 1940.0 2300.0

BSS ... 0. 0. 0. 0. 0 .0 .0

CROSS-SECTION NUMBER 2

XS(I) = .010 FSTG(I) = .00

HS ... 1529.0 1540.0 1560.0 1580.0 1600.0 1620.0 1640.0 1650.0

BS ... 300.0 450.0 630.0 840.0 1070.0 1600.0 1940.0 2300.0

0. 0. 0. 0. 0. 0. 0. 0. 0

CROSS-SECTION NUMBER 3

XS(1) = .170 FSTG(1) = .00

HS ... 1515.0 1520.0 1540.0 1560.0 1580.0 1590.0 1600.0 1610.0

BS ... 40.0 105.0 300.0 680.0 770.0 850.0 980.0 1190.0

0. 0. 0. 0. 0. 0. 0. 0

CROSS-SECTION NUMBER 4

XS(1) = .440 FSTG(1) = .00

HS ... 1490.0 1495.0 1500.0 1510.0 1520.0 1540.0 1560.0 1564.0

BS ... 13.0 35.0 165.0 250.0 315.0 432.0 560.0 620.0

BSS0 .0 .0 .0 .0 .0 .0

= (1)2X .00 .680 FSTG(I) =

1464.0 1470.0 1480.0 1490.0 1500.0 1510.0 1520.0 1540.0 B\$... 180.0 470.0

1060.0

1180.0 1230.0 1450.0

BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 6

xs(1) =.980 FSTG(1) = .00

1430.0 1460.0 1470.0 1480.0 1490.0 1500.0 1505.0 1540.0

100.0 5.0 276.0 506,0 680.0 960.0 2395.0

BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 7

= (1)2X.990 FSTG(I) = .00

1429.0 1460.0 1470.0 1480.0 1490.0 1500.0 1505.0 1540.0

5.0 100.0 276.0 356.0 506.0 680.0 960.0 2395.0

.0 BSS0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 8

XS(I) = 1.050 FSTG(I) = .00

HS ... 1424.0 1425.0 1430.0 1440.0 1450.0 1460.0 1470.0 1484.0

BS ... 19.0 39.0 180.0 263.0 637.0 350,0 785.0 978.0

BSS0 .0 .0 .0 .0 .0 .0 .0

XS(I) = 1.400 FSTG(I) = .00

HS ... 1395.0 1400.0 1410.0 1420.0 1425.0 1430.0 1440.0 1488.0 BS ... 29.0 65.0 240.0 420.0 510.0 590.0 695.0 1185.0 .0 BSS0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 10

XS(I) = 1.480 FSIG(I) = .00

HS ... 1390.0 1395.0 1400.0 1405.0 1420.0 1430.0 1440.0 1462.0 BS ... 21.0 30.0 55.0 170.0 290.0 380.0 455.0 590.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 11

XS(I) = 1.930 FSTG(I) = .00

HS ... 1360.0 1365.0 1370.0 1390.0 1400.0 1420.0 1440.0 1450.0 BS 18.0 41.0 125.0 ... 255.0 320.0 435.0 560.0 650.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 12

XS(1) = 2.200 FSTG(1) = .00

HS ... 1340.0 1345.0 1350.0 1360.0 1380.0 1390.0 1420.0 1430.0 28.0 BS 46.0 140.0 240.0 425.0 520.0 710.0 790.0 BSS0 .0 .0 .0 .0 .0 .0 .0

XS(1) = 2.330 FSTG(1) = .00

1330.0 1340.0 1350.0 1360.0 1380.0 1390.0 1400.0 1410.0 HS ... BS ... 50.0 90.0 700.0 330.0 470.0 790.0 845.0 1020.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 14

XS(1) = 2.340 FSTG(1) = .00

1329.0 1340.0 1350.0 1360.0 1380.0 1390.0 1400.0 1410.0 17.0 90.0 330.0 470.0 700.0 790.0 845.0 1020.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 15

XS(1) = 2.900 FSTG(1) = .00

HS 1300.0 1305.0 1310.0 1320.0 1330.0 1340.0 1360.0 1390.0 23.0 285.0 40.0 145.0 380.0 450.0 630.0 850.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 16

XS(I) = 3.000 FSTG(I) = .00

HS ... 1295.0 1325.0 1330.0 1340.0 1350.0 1360.0 1370.0 1380.0 BS ---30.0 250.0 540.0 830.0 985.0 1135.0 1290.0 1450.0 BSS0 .0 .0 .0 .0 .0 .0 .0

XS(1) = 3.010 FSTG(1) = .00

HS ... 1294.0 1325.0 1330.0 1340.0 1350.0 1360.0 1370.0 1380.0 BS ... 30.0 250.0 540.0 830.0 985.0 1135.0 1290.0 1450.0 BSS0 .0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION NUMBER 18

XS(1) = 3.280 FSIG(1) = .00

HS ... 1278.0 1280.0 1300.0 1320.0 1340.0 1360.0 1380.0 1400.0 BS ... 35.0 115.0 300.0 530.0 665.0 780.0 915.0 1095.0 BSS0 .0 .0 .0 .0 .0 .0 .0

CROSS-SECTION	1	BST		300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	- 1	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	2	BST		300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	2	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	3	BST		40.00	105.00	300.00	680.00	770.00	850.00	980.00	1190.00
CROSS-SECTION	3	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	4	BST		13.00	35.00	165.00	250.00	315.00	432.00	560.00	620.00
CROSS-SECTION	4	BSST		.00	.00	.00	.00	.00	.00	.00	-00
CROSS-SECTION	5	BST	49.47	5.00	10.00	180.00	470.00	1060.00	1180.00	1230.00	1450.00
CROSS-SECTION	5	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	6	BST		5.00	100.00	276.00	356.00	506.00	680.00	960.00	2395.00
CROSS-SECTION	6	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	7	BST		5.00	100.00	276.00	356.00	506.00	680.00	960.00	2395.00
CROSS-SECTION	7	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	8	BST		19.00	39.00	180.00	263.00	350.00	637.00	785.00	978.00
CROSS-SECTION	8	BSST		-00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	9	BST		29.00	65.00	240.00	420.00	510.00	590.00	695.00	1185.00
CROSS-SECTION	9	BSST		.00	-00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	10	BST		21.00	30.00	55.00	170.00	290.00	380.00	455.00	590.00
CROSS-SECTION	10	BSST		-00	.00	.00	.00	.00	.00	-00	.00
CROSS-SECTION	11	BST		18.00	41.00	125.00	255.00	320.00	435.00	560.00	650.00
CROSS-SECTION	11	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	12	BST		28.00	46.00	140.00	240.00	425.00	520.00	710.00	790.00
CROSS-SECTION	12	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	13	BST		50.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	13	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	14	BST		17.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	14	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	15	BST		23.00	40.00	145.00	285.00	380.00	450.00	630.00	850.00
CROSS-SECTION	15	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	16	BST		30.00	250.00	540.00	830.00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	16	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	17	BST		30.00	250.00	540.00	830.00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	17	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	18	BST		35.00	115.00	300.00	530.00	665.00	780.00	915.00	1095.00
CROSS-SECTION	18	BSST		.00	.00	.00	.00	.00	.00	.00	.00

CROSS-SECTION	1	BST		300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	1	BSST		.00	.00	.00	00	.00	.00	.00	.00
CROSS-SECTION	2	BST		300.00	450.00	630.00	840.00	1070.00	1600.00	1940.00	2300.00
CROSS-SECTION	2	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	3	BST		40.00	105.00	300.00	680.00	770.00	850.00	980.00	1190.00
CROSS-SECTION	3	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	4	BST		13.00	35.00	165.00	250.00	315.00	432.00	560.00	620.00
CROSS-SECTION	4	BSST		-00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	5	BST		5.00	10.00	180.00	470.00	970.00	1180.00	1230.00	1450.00
CROSS-SECTION	5	BSST	***	.00	.00	-00	.00	90.00	.00	-00	.00
CROSS-SECTION	6	BST		5.00	100.00	276.00	356.00	506.00	680.00	930.00	2395.00
CROSS-SECTION	6	BSST		.00	.00	.00	.00	.00	.00	30.00	.00
CROSS-SECTION	7	BST		5.00	100.00	276.00	356.00	506.00	680.00	930.00	2395.00
CROSS-SECTION	7	BSST		.00	.00	.00	.00	.00	.00	30.00	.00
CROSS-SECTION	8	BST	***	19.00	39.00	180.00	263.00	350.00	637.00	785.00	978.00
CROSS-SECTION	8	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	9	BST		29.00	65.00	240.00	420.00	510.00	590.00	695.00	1185.00
CROSS-SECTION	9	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	10	BST		21.00	30.00	55.00	170.00	290.00	380.00	455.00	590.00
CROSS-SECTION	10	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	11	BST		18.00	41.00	125.00	255.00	320.00	435.00	560.00	650.00
CROSS-SECTION	11	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	12	BST		28.00	46.00	140.00	240.00	425.00	520.00	710.00	790.00
CROSS-SECTION	12	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	13	BST		50.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	13	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	14	BST		17.00	90.00	330.00	470.00	700.00	790.00	845.00	1020.00
CROSS-SECTION	14	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	15	BST		23.00	40.00	145.00	285.00	380.00	450.00	630.00	850.00
CROSS-SECTION	15	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	16	BST		30.00	250.00	540.00	830.00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	16	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	17	BST		30.00	250.00	540.00	830,00	985.00	1135.00	1290.00	1450.00
CROSS-SECTION	17	BSST		.00	.00	.00	.00	.00	.00	.00	.00
CROSS-SECTION	18	BST		35.00	115.00	300.00	530.00	665.00	780.00	915.00	1095.00
CROSS-SECTION	18	BSST		.00	.00	.00	.00	.00	.00	.00	.00

CROSS-SECTION	1	HST		1530.00	1540.00	1560.00	1580.00	1600.00	1620.00	1640.00	1650.00
CROSS-SECTION	2	HST	***	1529.00	1540.00	1560.00	1580.00	1600.00	1620.00	1640.00	1650.00
CROSS-SECTION	3	HST		1515.00	1520.00	1540.00	1560.00	1580.00	1590.00	1600.00	1610.00
CROSS-SECTION	4	HST		1490.00	1495.00	1500.00	1510.00	1520.00	1540.00	1560.00	1564.00
CROSS-SECTION	5	HST		1464.00	1470.00	1480.00	1490.00	1500.00	1510.00	1520.00	1540.00
CROSS-SECTION	6	HST		1430.00	1460.00	1470.00	1480.00	1490.00	1500.00	1505.00	1540.00
CROSS-SECTION	7	HST		1429.00	1460.00	1470.00	1480.00	1490,00	1500.00	1505.00	1540.00
CROSS-SECTION	8	HST		1424.00	1425.00	1430.00	1440.00	1450.00	1460.00	1470.00	1484.00
CROSS-SECTION	9	HST		1395.00	1400.00	1410.00	1420.00	1425.00	1430.00	1440.00	1488.00
CROSS-SECTION	10	HST		1390.00	1395.00	1400.00	1405.00	1420.00	1430.00	1440.00	1462.00
CROSS-SECTION	11	HST		1360.00	1365.00	1370.00	1390.00	1400.00	1420.00	1440.00	1450.00
CROSS-SECTION	12	HST		1340.00	1345.00	1350,00	1360.00	1380.00	1390.00	1420.00	1430.00
CROSS-SECTION	13	HST		1330.00	1340.00	1350.00	1360.00	1380.00	1390,00	1400.00	1410.00
CROSS-SECTION	14	HST		1329.00	1340.00	1350.00	1360.00	1380.00	1390.00	1400.00	1410.00
CROSS-SECTION	15	HST		1300,00	1305.00	1310.00	1320.00	1330.00	1340.00	1360.00	1390.00
CROSS-SECTION	16	HST		1295.00	1325.00	1330.00	1340.00	1350.00	1360.00	1370.00	1380.00
CROSS-SECTION	17	HST		1294.00	1325.00	1330.00	1340.00	1350.00	1360.00	1370.00	1380.00
CROSS-SECTION	18	HST		1278.00	1280.00	1300.00	1320.00	1340 00	1360 00	1380 00	1400 00

REACH	1	050	.060	. 100	.120	. 150	.150	.180	.180
REACH	2	050	.060	.100	. 120	. 150	.150	. 180	.180
REACH	3	.050	.060	.100	.120	.150	.150	.180	.180
REACH	4	.050	.060	-100	.120	.150	.150	. 180	.180
REACH	5	. 055	.060	.100	.120	.150	. 150	.180	.180
REACH	6	. 060	.070	.100	.120	. 150	. 150	.180	.180
REACH	7	. 060	.070	.100	.120	.150	.150	.180	.180
REACH	8	.050	.060	.100	.120	.150	.150	-180	.180
REACH	9	.050	.060	.100	.120	.150	.150	.180	.180
REACH	10	.050	.060	.100	.120	-150	.150	.180	.180
REACH 1	t	.055	.060	.100	.120	.150	.150	.180	.180
REACH 1	2	-050	.060	.100	.120	.150	.150	.180	.180
REACH 1	3	.055	.060	.100	.120	.150	-150	.180	.180
REACH 1	4	.055	.060	.100	.120	.150	.150	.180	.180
REACH 1	5	.055	.060	.100	.120	. 150	.150	.180	.180
REACH 1	5	.055	.060	.100	.120	. 150	. 150	.180	.180
REACH 17		.055	.060	.100	.120	.150	. 150	.180	.180

CROSS-SECTIONAL VARIABLES FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER UNITS VARIABLE

MINIMUM COMPUTATIONAL DISTANCE USED BETWEEN CROSS-SECTIONS

MILE DXM(1)

CONTRACTION - EXPANSION COEFFICIENTS
BETWEEN CROSS-SECTIONS

FKC(I)

REACH NUMBER	DXM(I)	FKC(I)
1	.050	.000
2	.000	.000
. 3	.050	.250
4	.050	.000
5	.050	400
6	.000	.000
7	.050	.000
8	.000	.250
9	.050	400
10	.050	.250
11	.050	.000
12	.000	.000
13	.000	.000
14	.000	.000
15	.050	.000
16	.000	300
17	-000	.000

DOWNSTREAM FLOW PARAMETERS FOR Lake Petit Dam BELOW Emerg. Action Plan

PARAMETER *************************	UNITS A	ARIABLE	VALUE *******	
MAX DISCHARGE AT DOWNSTREAM EXTREMITY	CFS	QMAXD	.0	
MAX LATERAL OUTFLOW PRODUCING LOSSES	CFS /FEET	QLL	.000	
INITIAL SIZE OF TIME STEP	HOUR	DTHM	-27.0000	
DOWNSTREAM BOUNDARY PARAMETER	FEET	YDN	.000000	
SLOPE OF CHANNEL DOWNSTREAM OF DAM	FPM	SOM	.00	
THETA WEIGHTING FACTOR		THETA	.00	
CONVERGENCE CRITERION FOR STAGE	FEET	EPSY	.000000	
TIME AT WHICH DAM STARTS TO FAIL	HOUR	TFI	.00	

AT REACH= 3 DXM SHOULD BE CHANGED TO .043 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 5 DXM SHOULD BE CHANGED TO .009 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 7 DXM SHOULD BE CHANGED TO .001 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 8 DXM WAS CHANGED TO .048 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 9 DXM SHOULD BE CHANGED TO .017 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 12 DXM WAS CHANGED TO .039 DUE TO EXP/CONTRACT CRITERIA

2 DXM WAS CHANGED TO .016 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 14 DXM WAS CHANGED TO .166 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 15 DXM SHOULD BE CHANGED TO .004 DUE TO EXP/CONTRACT CRITERIA

AT REACH= 17 DXM WAS CHANGED TO .010 DUE TO EXP/CONTRACT CRITERIA

COMPUTATIONS WILL USE THE FOLLOWING DXM VALUES

101.000 .016 .050 .050 .050 101.000 .050 .048 .050 .050 .050 .039 101.000 .166 .050 101.000 .010

LATERAL INFLOW REACH NUMBER

LQX(I)

2 7

(QL(L, 1),L=1,ITEH) 50. 500. 1000.

AT REACH=

(QL(L, 2),L=1,ITEH) 0. 0. 1000.

RE-NUMBERED VALUES FOR IDAM

1DAM(1) = 1

IDAM(2) = 27

IDAM(3) = 54

IDAM(4) = 60

INITIAL CONDITIONS

INITIAL CONDITIONS

(QDI(I), I=1	(N)					
1100.	1100.	1150.	1150.	1150.	1150.	1150.	1150.
1150.	1150.	1150.	1150.	1150.	1150.	1150.	1150.
1150.	1150.	1150.	1150.	1150.	1150.	1150.	1150.
1150.	1150.	1027.	1027.	1027.	1027.	1027.	1027.
1027.	1027.	1027.	1027.	1027.	1027.	1027.	1027.
1027.	1027.	1027.	1027.	1027.	1027.	1027.	1027.
1027.	1027.	1027.	1027.	1027.	503.	503.	503.
503.	503.	503.	503.	503.	503.	503.	503.
503.	503.	503.	503.	503.	503.	503.	503.
503.	503.	503.	503.	503.	503.	503.	503.
503.	503.	503.	503.	503.	503.	503.	503.
	YI(I), I=1,		arts de	25.76	4000		
1648.00	1530.02	1528.68	1527.35	1526.04	1524.75	1523.48	1522,26
1521.12	1520.18	1519.40	1518.18	1513.41	1508.86	1504.18	1500.67
1496.62	1490.84	1485.01	1479.68	1473.72	1465.83	1464.26	1464.31
1464.31	1464.30	1464.30	1437.41	1428.06	1424-11	1420.04	1415.96
1411.50	1407.59	1402.37	1399.78	1395.64	1392.33	1389.00	1385.67
1382.34	1378.99	1375.64	1372.26	1368.80	1364.99	1360.75	1356.71
1352.32	1348.63	1343.63	1340.51	1335.86	1335.00	1332,67	1322.70
1313.26	1303.22	1300.44	1300.00	1296.67	1296.06	1295.46	1294.86
1294.26	1293.65	1293.05	1292.45	1291.84	1291.24	1290.64	1290.03
1289.43	1288.82	1288.22	1287.61	1287.00	1286.39	1285.78	1285.17
1284.55	1283.93	1283.31	1282.68	1282.03	1281.36	1280.65	1280.04

TIME PARAMETERS OF OUTFLOW HYDROGRAPH IMMEDIATELY DOWNSTREAM OF DAM

PARAMETER *********************************	UNITS	VARIABLE	VALUE *******
TIME TO FAILURE	HR	TFH	.500
TIME TO START OF RISING LIMB OF HYDROGRAPH	HR	TFO	.810
TIME TO PEAK	HR	TP	24.000
TIME STEP SIZE	HR	DTHI	.019

ROUTING COMPLETED.

KTIME= 139 ALLOWABLE KTIME= 699 TT= .

PROFILE OF CRESTS AND TIMES FOR Lake Petit Dam BELOW Emerg. Action Plan

DISTANCE FROM DAM	MAX ELEV	MAX FLOW	TIME MAX	MAX VEL	FLOOD ELEV	TIME FLOOD
MILE	FEET	CFS	ELEV-HRS	FPS	FEET	ELEV-HRS
*****	*****	******	****	*****	*****	*****
4.4	4130 00	10.000	312	4.55		
.000	1647.99	225253	.000	4.85	.00	.00
.010	1573.75	225253	.415	10.47	.00	.00
.026	1573.30	224588	.419	10.13	.00	-00
.042	1572.80	224134	.419	9.72	.00	.00
.058	1572.26	223785	.419	9.87	.00	.00
-074	1571.66	223415	.419	10.06	.00	.00
.090	1571.00	223027	.419	10.28	.00	-00
.106	1570.25	222618	.422	10.53	.00	.00
. 122	1569.42	222188	.422	10.84	.00	.00
. 138	1568.46	221823	.422	11.24	.00	.00
.154	1567.35	221441	.426	11.79	.00	.00
.170	1566.04	221077	.426	12.78	.00	.00
.224	1562.29	220031	.430	12.07	.00	.00
.278	1558.11	219241	.430	11.86	.00	.00
.332	1553.61	218685	.433	12.35	.00	.00
.386	1548.18	218370	,437	13.56	.00	.00
-440	1538.49	218198	.437	17.69	.00	.00
.500	1530.55	217791	.448	14.21	.00	.00
.560	1524.87	216161	.470	11.78	.00	.00
.620	1521.66	212547	.481	9.53	.00	-00
.680	1519.81	207154	.485	8.04	.00	.00
.730	1518.17	202213	.493	15.41	.00	.00
.780	1516.32	198214	.496	678.21	.00	.00
.830	1514.17	194569	.500	23.92	.00	.00
.880	1511.56	191643	.504	12.35	.00	.00
.930	1508.17	189187	.507	14.82	.00	.00
.980	1503.47	188110	.511	52.90	.00	-00
.990	1498.02	188110	.515	*****	.00	.00
1.050	1440.73	187902	.504	263.91	.00	.00
1,100	1474.10	187009	.533	33.16	.00	.00
1.150	1472.01	185210	.537	98.88	.00	.00
1.200	1429.45	184534	.511	64.78	.00	.00
1.250	1460.64	183271	.556	9.45	.00	.00
1.300	1458.53	180525	.563	9.61	.00	.00
1.350	1456.87	177649	.567	9.04	.00	.00
1.400	1455.56	174969	.567	6.53	.00	.00
1.480	1451.95	172268	-574	10.01	.00	.00
1.530	1448.51	171083	.581	10.35	.00	.00
1.580	1444.83	169890	.585	10.68	.00	.00
1.630	1440.84	168703	.589	10.98	.00	.00
1.680	1436,56	167644	.593	11.28	.00	.00
1.730	1432.29	166790	.596	11.58	.00	.00
1.780	1428.02	166109	.600	11.90	.00	.00
1.830	1423.73	165584	.604	12.24		.00
1.880	1419.42	165159	.607	12.63	.00	-00
1.930	1414.63	164765	.611	13.22	.00	.00
1.984	1409.65	164317	.619			
2.038	1404.72		.622	12.95	.00	.00
2.036	1404.72	163797	-022	12.67	,00	.00

PROFILE OF CRESTS AND TIMES FOR Lake Petit Dam BELOW Emerg. Action Plan

DISTANCE						
FROM DAM	MAX ELEV	MAX FLOW	TIME MAX	MAX VEL	FLOOD ELEV	TIME FLOOD
MILE	FEET	CFS	ELEV-HRS	FPS	FEET	ELEV-HRS
******	****	****	*****	*****	****	****
2.092	1399.91	163110	.633	12 (0	.00	.00
2.146	1395.38	162188	.641	12.40	.00	.00
2.200	1391.28	160903	.648	11.97	.00	.00
2.243	1389.18	159642	.652	10.28	.00	
2.287	1387.74	158460	.656	8.44	.00	.00
2.330	1386.73	157479	.656	6.90	.00	.00
2.340	1379.03	157479	.674	9.62	.00	.00
2.527	1369.62	151707	.696	9.02	.00	7.5
2.713	1361.17	147058	.715	9.27	.00	.00
2.900	1350.53					.00
2.950	1348.55	144487 144036	.730	9.83	.00	.00
3.000			.730	7.36	.00	.00
	1345.94	143793	.733	7.93	.00	.00
3.010	1324.73	143793	.622	50.52	.00	.00
3.020	1339.38	143748	.748	21.29	.00	.00
3.030	1338.41	143615	.752	20.58	.00	-00
3.040	1337.45	143469	.752	20.11	.00	.00
3.050	1336.51	143334	.756	19.75	.00	.00
3.060	1335.59	143185	.756	19.36	.00	.00
3.070	1334.69	143040	.759	19.01	.00	.00
3.080	1333.80	142892	.759	18.64	.00	.00
3.090	1332.93	142730	.763	18.24	.00	.00
3.100	1332.08	142582	.763	17.77	.00	.00
3.110	1331.25	142422	.767	17.42	.00	.00
3.120	1330.44	142254	.767	16.96	.00	.00
3.130	1329.64	142096	.770	16.51	.00	.00
3.140	1328.86	141924	.770	16.04	.00	.00
3.150	1328.09	141751	.774	15.57	.00	.00
3.160	1327.34	141583	.774	15.06	.00	.00
3.170	1326.60	141402	.778	14.56	.00	.00
3.180	1325.87	141225	.778	14.05	.00	.00
3,190	1325.16	141048	.781	13.57	.00	.00
3.200	1324.45	140859	.781	13.20	.00	.00
3.210	1323.76	140678	.785	12.98	.00	.00
3.220	1323.07	140494	.785	12.82	.00	.00
3,230	1322.38	140299	.789	12.82	.00	.00
3.240	1321.70	140113	.789	12.83	.00	.00
3.250	1321.02	139923	.793	12.85	.00	
3.260	1320.34		.795	12.90		.00
	150000000000000000000000000000000000000	139722			.00	.00
3.270	1319.66	139533	.796	12.98	.00	.00
3.280	1318.98	139337	.800	13.10	.00	.00

	PEAK EL	EVA1	TION PROFILE		MILES						ELEV FEET	MILE
	.0	.3	.7	1.0	1.3	1.6	2.0	2.3	2.6	3.0	3.3	
0	1647.99*										.1648.0	.0
					2				4			
								•		•	•	
		-	4.0									
									:			
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	1548.18.	. *			Ye.						.1548.2	.4
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		10		*.		•					.1503.5	1.0
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			*			*		*	100		.1452.0	1.5
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	14401211					* .					. 1446.5	1.6
						*.					.1440.8	1.6
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		0.90	-									
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	1414.63.						10.00				*****	
	1414.03.	150					*		•		.1414.6	1.9
		12								7.		2.0
			1.4				. *				1404.7	2.0
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		F	141				. *				.1399.9	2.1
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	1369.62.	- 1			32		4		*		.1369.6	2.5
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	1334.69.							3		. *	.1334.7	3.1
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	1326.60.		146							. *	.1326.6	3.2
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4								200			-113	

	PEAK DIS	CHARGE	PROFILE	МІ	LES						19	MILE	
	-0	.3	.7	1.0	1.3	1.6	2.0	2.3	2.6	3.0	3.3	CFS	
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+	*		•						•	(·		224135.	0/
	.*											223785.	.04
+			ý.										
+		1										223027.	.09
	*			,						9		222189.	.12
+	221441 *							9				221441.	. 15
+	*											261441.	. 13
			,					4	1.0				52
		*				*	1.2					220031. 219241.	.22
		*										218686.	.28
+		.*											
								1				217792.	.50
				4.	1.6								
	8		* .			4			1.0		4	216162.	.56
			1		1.0						16		
		- 1	1										
	1										•	212548.	.62
		- 2											
			*				1.6						
										4	-		
	207155		*	•							*	207155.	.68
							- 3						
						1.00		- 1					
	•	•											
					7	7	*	0.					
									- 1			202213.	.73
									100				
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	(3)		* *									198214.	.78
										•			
	4												
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				4									
				*		21						Sino	2.5
					•							191644.	.88
						- 1							
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	189187			*					1.30	•		189187. 188111.	.93 .98
+	2.5			*									
	,			. *				4			4	187903.	1.05
												187009.	1.10
	185211				*	1						185211.	1.15
		1.			*							184534.	1.15
	100									•			
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			*0		*.							180526.	1.30
D		9		27		•						419.	
1					. *			1.67				177649.	1.35

S								- 1		4.0		
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	174707			•							174969.	1.40
G E			- 2									
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	•					*					172268.	1.48
		- 1							•		171083.	1.53
			141			* .					169891.	1.58
		4				*.					168704.	1.63
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		- 5					*				165584.	1.83
+	**********		Y				* .					
	164766	•				•	* :				164766.	1.93
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						1.0		*	1.0		162189.	2.15
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			- 0								139042.	2.24
	6						4.	*.			158461.	2.29
	ş.							*			157480.	2.29
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	151708	1							*		151708.	2.53
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ELI	MILE	HOUR							IILES	,			
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			*.		9.5	*95	20					•	
			* :	•	(4)	•	900	•0		6 *	1.0	•	
	3.2	-8).*					1				3
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			* .	8*		•	8	1.5		*	141	*	*
			* *	*	5.5	1.81	•	0.5					1
			*	:			(3*8	e:	9	(*)		
			* .							2	-	¥0.	*
			* 6		3.450	3.63			5 3		•	*5	
	3.1	.8	*	12					•	•	•	•	
1328			*						25	2	%	172	2
			*			1983 U#0						192	
								80	•		*	3901	•
			k (e)			59.5	•		•	*			T-1
			235	° 4	•	300		30.1	200	*:		•	.76.
4777	3.1	. 8	•	* *	•	•					•	(₩)	.70.
1333			090	. *								6.7	
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1379	2.3	.7		*		**	1/1	*			•		•
13/7	2.3	.7	12	27	2	_*	185		2			*	
1386		2.0	05	50									
			5.			W.		*	*			•	·-
4700	2.2	.7	4		*	* -			•	*	300	*:	65.
1389						*				5		20	2
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	2.1	.6			8		. *		¥	¥		*6	
1399													
472/	3.0	.6		.*		*	(00)	*	*				*:
1324							*	8	2	2	2	16	20
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	1.9	.6		0.00		â.	*		*	*		0.00	
1414													
			*		•5	**	W 187	183	*:	8			
	1.8	.6	•	•	•	** -0		. *		*	3	385	50.
1428	1.0	.0	•	(*)	55			•					
				•	*	•:	9	*	•		3	2.0	
				(*)	•	•		*			*		1060
		,		3.0	•	•		*		•	*		
1444	1.6	.6	**	•	•)	•			. w.	85		if	0.00
1444			Į.				÷	w 120	*	27	4	3	
			*	3.5	(*)	(#)	ï	(*)	. *				•
	1.4	.6	•			V#3	Si .	o * 0.	· *	100	*		3.
1455									*				
			*	1011	(*)	50±6	36	3 6 2.	w "	1.65	*	1	8
		.6	•		:		•		* :		÷		6.
	1.2	, D											

GAGE ZERO = 1530.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1647.98 FEET FLOOD STAGE NOT AVAILABLE

MAX STAGE = 117.98 FEET AT 1 MAX FLOW = 225253 CFS AT 1

AT TIME = .000 HOURS AT TIME = .389 HOURS

		PINA I	- 40.	LLJEJJ	Cra	Α1	CIP			» n	COKS	
TIME	STAGE	FLOW										
HR	FEET	CFS	0	50000	10	0000	150	000	2000	00	25	0000
.00	118.0	1100	*				20.7					
.02	118.0	1400	*									
.04	117.9	2635	*					1				
.06	117.9	5366	*									
.08	117.8	9664	. *									
.10	117.7	15858	. *									
.12	117.4	24096		*								
.14	117.1	34207		* .								
.16	116.6	46266		*.		1				1		
.18	116.0	60097		. *								
.20	115.2	75855			*							
.22	114.1	93104				*.						- 1
.24	112.8	111586				. *						
.26	111.2	130717					*					
.28	109.3	150167						*				
.30	106.9	169111							*			
.32	104.1	186788							*			
.34	100.8	202388								*		
.36	96.9	214908				4					W	
.38	92.3	222881								0	*	
.40	86.7	224884									*	- 2
.42	80.2	220766									*	
.44	72.3	206897								. *		
.46	62.3	180378							*			
.48	49.2	136350					*					
.50	33.6	54294		. *								
.52	22.6	14978	. *									- 0
.54	15.7	7954	. *									
.56	9.9	3870	*									
.58	5.7	1660	*									
.60	3.4	1100	*									
.62	1.6	1100	*									
.64	.7	1100	*									
.66	.7	1100	*									
.68	.7	1100	*	15				-		3.		
.70	.7	1100	*					2				
.72	.7	1100	*							5		
.74	.7	1100	*									
.76	.7	1100	*							4		
.78	.7	1100	*					1		3		
.80	.7	1100	*									

1529.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1573.75 FEET
FLOOD STAGE NOT AVAILABLE
MAX STAGE = 44.75 FEET AT TIME = .415 HOURS
MAX FLOW = 225253 CFS AT TIME = .389 HOURS GAGE ZERO = 1529.00 FEET

		MAX F	LUM =	225253	:FS	AT TIME :	= .389 H	DURS
TIME	STAGE	FLOW						
HR	FEET	CFS	0	50000	100000	150000	200000	250000
.00	1.0	1100	*		70117		200000	230000
.02	1.2	1400	*			- 3	5	
-04	1.7	2635	*					
.06	2.6	5366	*	- 3				
.08	3.8	9664			- 3		•	
.10	5.2	15858	. *		- 12			
.12	6.9	24096		*				
.14	9.1	34207		*				
.16	12.0	46266		*.	- 3			
.18	15.6	60097	1.5	. *				
.20	19.5	75855			*			
.22	23.5	93104			*			4
.24	27.1	111586	2	- 3	100	*		
.26	30.5	130717				*		
.28	33.5	150167						
.30	36.1	169111			1.0		*	
.32	38.5	186788						
.34	40.6	202388						
.36	42.4	214908						2
.38	43.7	222881						7
.40	44.5	224884						
.42	44.7	220766						
-44	44.2	206897		-			1.	
.46	42.4	180378						
.48	39.0	136350						
.50	31.5	54294	•	-				
.52	22.7	14978		1.00				
.54	16.0	7954						
.56	10.5	3870				7.		
.58	5.9	1660						
.60	2.5	1100	*					
.62	1.0	1100				1.00		
-64	1.0	1100						
.66	1.0	1100	-					
.68	1.0		4					
.70	1.0	1100	-					
.72		1100	-					
	1.0	1100	2				1.0	
.74	1.0	1100				4.		4
.76	1.0	1100	*				60	
.78	1.0	1100	-	- 0				
.80	1.0	1100	-	*				1.00

GAGE ZERO = 1430.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1503.47 FEET FLOOD STAGE NOT AVAILABLE

		-	1101 711	LILE CHURCH			
MAX	STAGE =	73.47	FEET	AT TIME	=	.511 HOURS	
MAX	FLOW =	188111	CFS	AT TIME	=	.507 HOURS	

TIME	STAGE	FLOW										
HR	FEET	CFS	0		50000	1	00000	1500	000	2000	100	250000
.00	33.4	2613		*						-		75 1111
.02	30.3	5121		*								
.04	25.7	4811	. 1	*								
.06	23.6	2317	*									
.08	20.8	2631		k			- 1					2.
.10	16.6	2779		ie .								1
.12	13.6	1726	*		4						120	
-14	11.6	1523	*						-			
.16	10.0	1801	*				- 2					
.18	29.0	69005				*						
.20	43.8	15079		*							-	
.22	42.4	16540		*					-			
.24	44.2	17191		*	1							- 5
.26	45.8	23296			*		- 2					- 0
.28	47.6	31563			*						Ů.	
.30	49.5	41665			*		20					3.5
.32	51.7	54403			.*		- 1				3	
.34	54.6	71766				*					3	
.36	58.3	94191					*		18.		2	
.38	62.5	118573						*				
.40	66.0	139640						*				
.42	68.6	155628	9						*			
.44	70.3	166552	-							*		
.46	71.6	175582			100					*		
.48	72.7	183127								*		•
.50	73.4	187740	3		7					*		
.52	73.4	186728								*		
.54	72.5	178620								*		*
.56	70.7	164887							. 1	*		
.58	68.4	148320	0						*			•
.60	65.1	126451						*				•
.62	61.6	106542			- 7		. w		,			
.64	58.2	89562	•									
.66	55.3	76051	•		•		C.C.		5			
.68	52.5	65301			1.1	. "						
.70	50.0	55438										
.72	47.4	45864							7			
.74	44.6	36657										
.76	41.7	28345										
				-								
-78	38.6	21912									•	
.80	35.7	17148		*								

GAGE ZERO = 1330.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1386.73 FEET

FLOOD STAGE NOT AVAILABLE

MAX STAGE = 56.73 FEET AT TIME = .656 HOURS

MAX FLOW = 157480 CFS AT TIME = .644 HOURS

					20.2				3.11.2	
TIME	STAGE	FLOW								
HR	FEET	CFS	0	50000	1000	00	150000	200000	250000	
.00	7.5	915	*	30000	,000	00	130000	200000	2,0000	
.02	8.6	1132	*							
.04	9.1	1227	*							
.06	9.3	1286	*	-		3				
.08	9.5	1326	*							
.10	9.6	1330	*							
.12	9.5	1326	*							
.14	9.5	1325	*							
.16	9.5	1313	*							
.18	9.5	1326	*			•				
.20	9.7	1351	*							
.22	9.7	1353	skr							
.24	9.8	1373	*	•						
.26	9.9	1393	*							
.28	10.0	1420	*							
.30	10.2	1471								
.32	10.8	1600	*							
.34	12.1	1889	*							
.36	14.3	2430	-	1		5				
							*			
.38	17.3	3846								
.40	20.7	7632							14.7	
	23.8	13680								
-44	26.7	21861							4.1	
.46	29.6	33046		* .						
.48	32.9	47341		*, ·					1,000	
.50	36.7	64808			200		1.0			
-52	41.0	84332			*					
.54	45.3	104049							1967	
.56	49.0	121940				. "			1.0	
.58	51.9	136965					*			
.60	54.2	147251					*.		7	
.62	55.7	154076				•	.*	40	1.0	
.64	56.5	157351					.*			
.66	56.7	156324	•				.*			
.68	56.4	151808				•	*			
.70	55.6	144971					*.			
.72	54.6	136606				*	*			
.74	53.3	127580				1	*			
.76	51.8	118358				. *				
.78	50.2	109405		-		. *	D(D		4	
.80	48.6	100733				*				

GAGE ZERO = 1295.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1345.94 FEET

FLOOD STAGE NOT AVAILABLE

MAX STAGE = 50.94 FEET AT TIME = .733 HOURS

MAX FLOW = 143793 CFS AT TIME = .733 HOURS

		MAA F	LOW	-	143/5	/3 (FS	AI	TIME	=	.733 HC	OURS
TIME	STAGE	FLOW										
HR	FEET	CFS	0		50000)	10000	0	15000	0	200000	250000
.00	5.0	505	*						10000			
.02	5.0	507	*								100	
.04	5.1	517	*									
.06	5.3	550	*									- 2
.08	5.7	615	*									
.10	6.3	718	*				1.5					
.12	7.0	837	*									
.14	7.6	951	*									
.16	8.1	1045	*									
.18	8.5	1121	*									
.20	8.8	1178	*		-							
.22	9.0	1219	*						- 0			
.24	9.2	1247	*				- 17		100			
.26	9.3	1267	*									
.28	9.3	1282	w								- 3	
.30	9.4	1296	*						- 10			
.32	9.5	1310	*									
.34	9.5	1324	*								- 3	•
.36	9.6	1339	*								- 2	
.38	9.7	1353	*				- 1				- 2	
.40	9.7	1365	*		- 10		· ·		- 1			
.42	9.9	1392	*									
.44	10.2	1472	*						- 0		- 2	
.46	10.3	1491	*		- 6		- 1					
.48	10.1	1451	*		1.0				1 0			
.50	12.6	2011	*		- 3				- 1			
.52	19.5	3879	*				- 2					
.54	27.2	6399	*								,	
.56	34.3	15838		*								
.58	39.5	35940			*							
.60	43.8	60671				*	1					
.62	46.7	86636					*					
.64	48.2	110188					100	*				
.66	48.6	127851					- 10		*		- 5	
.68	49.7	135012			- 1		- 6		*			
.70	50.5	140586	2.						*			•
.72	50.9	143373							*			•
.74	50.9	143598	2				- 3		*			
.76	50.6	141501	•				- 3		*			
.78	50.0	137503							*		,	
.80	49.3	132271	31				(*		*			
.00	47.5										2	

GAGE ZERO = 1278.00 FEET MAX ELEVATION REACHED BY FLOOD WAVE = 1318.98 FEET

FLOOD STAGE NOT AVAILABLE 40.98 FEET MAX STAGE = AT TIME = .800 HOURS MAX FLOW = 139337 CFS AT TIME = .767 HOURS TIME STAGE FLOW CFS 0 50000 HR FEET 100000 150000 200000 250000 .00 505 2.0 .02 2.0 505 2.0 506 .04 .06 2.0 508 .08 2.0 517 2.0 531 .10 .12 2.1 574 .14 2.2 649 2.3 754 .16 .18 873 2.4 .20 980 .22 2.7 1073 .24 2.7 1141 .26 1195 .28 2.8 1232 2.9 .30 1257 .32 1276 2.9 .34 1291 .36 2.9 1305 .38 1320 .40 2.9 1334 .42 1348 3.0

.44 3.0 1362 .46 3.0 1395 .48 3.1 1447 .50 3.1 1473 .52 3.1 1532 -54 3.9 2442 .56 5.6 4916 .58 9.5 13544 30856 .60 15.4 .62 20.6 48247 24.9 66440 .64 93452 .66 30.2 .68 34.0 113083 .70 36.4 124862

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* U.S. ARMY CORPS OF ENGINEERS
HYDROLOGIC ENGINEERING CENTER
609 SECOND STREET
DAVIS, CALIFORNIA 95616
(916) 756-1104

Full Microcomputer Implementation
by
Haestad Methods, Inc.

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC16S, HEC1DB, AND HEC1KM.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRANTY VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

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107	0.0220 0.02321 0.02427 0.02565 0.02688 0.03847 0.0028 0.01643 0.01962
108	0.03581 0.03712 0.03825 0.03078 0.0447 0.0279 0.03083 0.03192
109	0.0480 0.04941 0.05084 0.05229 0.05724 0.04113 0.04546 0.04583 0.04522
110	0630 0.06461 0.06624 0.06789 0.06956 0.0725 0.02364 0.03964
111	0800 0.08181 0.08364 0.08540 0.08734 0.0005 0.0014 0.00590 0.00594
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113	0.1200 0.12225 0.12440 0.12705 0.12040 0.12225 0.11349 0.11364
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115	810 0.18512 0.18948 0.19408 0.19892 0.20400 0.20400 0.21500 0.20400
116	0.26092 0.27148 0.28300 0.30684 0.35434 0.43479
117	0.71304 0.72516 0.73500 0.74344 0.75136 0.75876
118	0.7720 0.77796 0.78364 0.78904 0.79416 0.79900 0.80360 0.80800 0.81220
119	0.8200 0.82367 0.82726 0.83079 0.83424 0.83763 0.84094 0.84419 0.84736
120	0.8535 0.85647 0.85936 0.86219 0.86494 0.86763 0.87024 0.87279 0.87526
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124	0.9377 0.93929 0.94080 0.94229 0.94375 0.94519 0.94660 0.94799 0.94935
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126	0.9647 0.96600 0.96724 0.96848 0.96971 0.97094 0.97216 0.97338 0.97459
127	0.9770 0.97820 0.97939 0.98058 0.98176 0.98294 0.98411 0.98528 0.98644
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U.S. ARMY CORPS OF ENGINEERS HYDROLOGIC ENGINEERING CENTER

609 SECOND STREET DAVIS, CALIFORNIA 95616 (916) 756-1104

Big Canoe - PMP Flood Hydrographs

OUTPUT CONTROL VARIABLES

IPRNI 4 PRINT CONTROL

IPLOT 0 PLOT CONTROL

QSCAL 0. HYRROGRAPH PLOT SCALE

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QSCAL 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

MINUTES IN COMPUTATION INTERVAL NUMBER OF HYDROGRAPH ORDINATES STARTING DATE STARTING TIME ENDING TIME CENTURY MARK ENDING DATE 9 0654 19 0000 HYDROGRAPH TIME DATA NDDATE MMIK IDATE ITIME 2 NOTIME ICENT

COMPUTATION INTERVAL 0.10 HOURS
TOTAL TIME BASE 6.90 HOURS

ENGLISH UNITS

DRAINAGE AREA

SQUARE MILES

PRECIPITATION DEPTH INCHES

LENGTH, ELEVATION FEET

FLOW

STORAGE VOLUME

SURFACE AREA

TEMPERATURE

DEGREES FAHRENHEIT

4 KK * PT24 * Pettit Lake 24-Hour PMP * * * * * *

5 KO OUTPUT CONTROL VARIABLES

1PRNT 4 PRINT CONTROL

1PLOT 0 PLOT CONTROL

QSCAL 0, HYDROGRAPH PLOT SCALE

8 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 6 TIME INTERVAL IN MINUTES

7 PB STORM 41.00 BASIN TOTAL PRECIPITATION 7 PB STORM 41.00 BASIN TOTAL PRECIPITATION 9 P1 INCREMENTAL PRECIPITATION PATA 7 PB STORM 41.00 BASIN TOTAL PRECIPITATION 9 P1 INCREMENTAL PRECIPITATION PATEN 9 P1 INCREMENTAL PRECIPITATION 9 P1 INCREMENTAL PRECI	CHARACTERISTICS REA 1.53 SUBBASIN AREA ATION DATA NATION DATA SHTAL PRECIPITATION PATTERN SHATE 1.33 INITIAL ABSTRACTION SHATE SHATE		BASIN RUNDFF DATA	RUNOFF	DATA									
PRECIPITATION DATA STORM 41.00 BASIN TOTAL PRECIPITATION STORM 41.00 BASIN TOTAL PRECIPITATION INCREMENTAL PRECIPITATION PATTERN 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	7 PB STORM 41.00 BASIN TOTAL PRECIPITATION 9 PJ INCREMENTAL PRECIPITATION PATTERN 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	6 BA	SUBBAS	De	ACTERISTIC: 1.53		N AREA							
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SUBBASIN CHARACTERISTICS

BA 38

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Cance Property Owner's Association for incorporation into this Emergency Action Plan.

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821.	197.	32.	6.		
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189.	. 509	.56	16.	2.	
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Pettit Lake 6-Hour PMP × 36

TIME INTERVAL IN MINUTES STARTING DATE STARTING TIME PRINT CONTROL PLOT CONTROL HYDROGRAPH PLOT SCALE SERIES TIME DATA FOR INPUT TIME SE JXMIN 1 TI JXDATE 1 0 ST JXTIME 0 ST OUTPUT CONTROL VARIABLES
IPRNT 4 PM
IPLOT 0 PI
OSCAL 0. H) 37 KO 콢 40

SUBBASIN CHARACTERISTICS SUBBASIN RUNDFF DATA

BA

38

Note: Inundation analysis was performed by Jordan, Jones, and Goulding and provided by the Big Canoe Property Owner's Association for incorporation into this Emergency Action Plan.

AREA
SUBBASIN
1.53

TAREA

ECIPITATION DATA

39 PB	STORM	30.20	BASIN TOTAL PRECIPITATIO	TAL PREC	IPITATION				
41 PI		PRECIPITAT	TON PATTE	RN					
		0.01	0.01	0.01	0.01	0.01	0.01	0.01	0 01
		0.01	0.01	0.01	0.02	0.02	0 02	20.0	200
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> 31. 99. 189. 313. 475 END-OF-PERIOD ORDINATES 751. 684. 604. 504. 405. 333. 276. 230. 197. 137. 113. 95. 80. 66. 55. 45. 38. 32. 22. 19. 16. 13. 11. 9. 8. 7. 6.

0.85 LAG

DIMENSIONLESS UNITGRAPH

SCS

9

29

CURVE NUMBER
PERCENT IMPERVIOUS AREA

0.00

SCS LOSS RATE STRTL CRVNBR RTIMP

ST 99

INITIAL ABSTRACTION

164. 27. 5. 68 KK * SC24 * Sconti Lake 24-Hour PMP

69 KO OUTPUT CONTROL VARIABLES
IPRNT 4 PRINT CONTROL
IPLOT 0 PLOT CONTROL
GSCAL 0. HYDROGRAPH PLOT SCALE
72 IN TIME DATA FOR INPUT TIME SERIES

TIME DATA FOR INPUT TIME SERIES
JXMIN 6 TIME INTERVAL IN MINUTES
JXDATE 1 0 STARTING DATE
JXTIME 0 STARTING TIME

SUBBASIN RUNOFF DATA

PRECIPITATION DATA

SUBBASIN CHARACTERISTICS TAREA 3.30

BA

20

SUBBASIN AREA

PRECIPITATION DATA

BB

7

STORM 41.00 BASIN TOTAL PRECIPITATION

安全市 安全市 安安市 安安市 安安市

73 PI	INCR	CREMENTAL 0.00 0.00 0.00 0.00 0.00 0.00	PRECIPITATION PATIERN 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	10N PATTEL 0.00 0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00 0.00 0.00	00000000	0.00	0.00	0.00	000000000000000000000000000000000000000	0.00	
ST 86	07 838	LOSS RATE STRTL CRVNBR RTIMP	1.33	INITIAL ABST CURVE NUMBER PERCENT IMPE	INITIAL ABSTRACTION CURVE NUMBER PERCENT IMPERVIOUS AREA	IREA						
an 66	SCS DI	IMENSION	DIMENSIONLESS UNITGRAPH TLAG 1.66 LA	APH								

	16. 680. 849. 356. 138. 53. 51. 21. 21.	39. 766. 810. 324. 127. 127. 19. 8.	77. 826. 768. 294. 116. 44. 17.	122. 877. 724. 266. 105. 40. 16. 7.	88	UNIT HYDROGRAPH 171. 234. 171. 926. 170. 926. 171. 926. 172. 926. 173. 611. 174. 13. 174. 13. 174. 13. 175. 10.	79. 55. 5. 5.	390. 927. 487. 187. 71. 27. 11.	485. 917. 433. 170. 25. 4.	390. 485. 588. 927. 917. 884. 487. 433. 393. 187. 170. 154. 71. 65. 59. 27. 25. 23. 11. 10. 9. 4. 4. 3.		
	在存在在在在在在在	***										
100 KK	* SC06	* * * *	Sconti Lake 6-Hour PMP	e 6-Hour F	N/P							
101 ко	TUTFUT I	I CONTROL IPRNT IPLOT QSCAL	VARIABLES 4 0 0	PRINT CONTROL PLOT CONTROL HYDROGRAPH PL	PRINT CONTROL PLOT CONTROL HYDROGRAPH PLOT SCALE	щ						
N 104 IN	TIME DO	DATA FOR INPUJXMIN JXDATE JXTIME N RUNOFF DATA	INPUT TIME 1 0 DATA	SERIES TIME INTERVAL STARTING DATE STARTING TIME	Z.	MINUTES						
102 BA	SUBBAS	ISIN CHARACTER TAREA IPITATION DATA	SUBBASIN CHARACTERISTICS TAREA 3.30 PRECIPITATION DATA	SUBBASIN AREA	AREA							
103 PB		STORM	30.20	BASIN TOT	BASIN TOTAL PRECIPITATION	ATION						
105 P.I	INCR	REMENTAL 0.01	PRECIPITATION PATTERN 0.01 0.01	O.01	0.01	0.01	0.01	10.0	0.01	0.01	0.01	

		0.01	0.01	0.01	0.01	0.02	0.05	0.02	50.03	50	02.0
		0.08	0.04	0.03	0.02	0.02	0.02	0.7	100	10.0	100
		0.01	0.01	0.01	0.01	0.01	0.01		0.01	0.0	500
		00.00	0.00	0.00	0.00	0.00	0.00	0.00	00.0	000	0.0
		00.0	0.00	0.00	00.00	0.00	0.00	00.00	00.0	200	900
		0.00	0.00	0.00	00.00	0.00	00.00	0.00	00.0	0.00	2
130 LS	SCS	SCS LOSS RATE									
		CRVNBR	1.33	CURVE NUMBER	INITIAL ABSTRACTION	*					
		RTIMP	0.00	PERCENT	PERCENT IMPERVIOUS AREA	AREA					
131 UD	SCS	SCS DIMENSIONLESS	_	APH							
		TLAG	1.66	LAG							
						1					
					N	UNIT HYDROGRAPH	APH				
					85 END-	PERIOD	ORDINATES				
	16.	39.	.11	122.		234.	303.			588	
	680.	766.	826.	877.			932.			884	
	849	810.	768.	724.			546.			393	
	356.	324.	294.	266.	245.	225.	205	187.	170.	154.	
	138.	127.	116.	105.			79.			59	
	53.	.84	44.	.07			30.			23	
	21.	16.	17.	16.			12.			6	
	6	80	7.	7.			5.			M	
	2.	2.	1.	1.							

RUNOFF SLUMMARY FLOW IN CUBIC FEET PER SECOND TIME IN HOURS, AREA IN SQUARE MILES

TIME OF MAX STAGE	1.53	1.53	3.30	3.30
MAXIMUM STAGE	3657.	3354.	5848.	6941.
BASIN	3657.	3354.	5848.	6941.
AVERAGE FLOW FOR MAXIMUM PERIOD	4203.	3857.	. 4229	7982.
LOW FOR M.	9.90	2.90	06.9	3.80
AVERAGE F	7100.	14624.	13294.	18830.
TIME OF PEAK	PT24	PT06	SC24	9008
PEAK FLOW				2
STATION 72-HOUR				
OPERATION 24-HOUR	HYDROGRAPH AT	HYDROGRAPH AT	HYDROGRAPH AT	HYDROGRAPH AT
OUR.				

*** NORMAL END OF HEC-1 ***

APPENDIX C DAM FEATURES AND PHOTOGRAPHS



N:B\Big Canoe\Projects\TN6338 - 2017 EAP Lake Petit Dam\G\S\MXD\Dam_Features.mxd 5/16/2017 8:22:44 AM

Geosyntec Consultants

PHOTOGRAPH LOG

: Emergency Action Plan for Dam Failure, Lake Petit Dam	PROJECT NAME: Emergency
---	-------------------------

CLIENT: Big Canoe Property Owner's Association

PROJECT NO.: TN6338 FILE NAME: AppD_Photos.ppt



Photograph 1: View of the low-level valve box



Photograph 2: View of the low-level valve box.

Geosyntec b

PHOTOGRAPH LOG

PROJECT NAME: Emergency Action Plan for Dam Failure, Lake Petit Dam	PROJECT NO.: TN6338
CLIENT: Big Canoe Property Owner's Association	FILE NAME: AppD Photos.ppt



Photograph 3: Tail Water Creek, 2016 - Low-level drain outlet.



Photograph 4: Spillway, 2016 – general view with moderate flow during observation

APPENDIX D EAP DISTRIBUTION AND ACCEPTANCE

APPENDIX D EAP DISTRIBUTION

The following entities maintain a copy of the EAP. Contact information can be found on Figure 1. The EAP should be signed by all parties, including those listed below, involved in plan implementation on the following pages.

- 1. Big Canoe Property Owner's Association
- 2. Big Canoe Public Safety Director
- 3. Pickens County Emergency Management Agency (EMA)
- 4. Dawson County 911 Service
- 5. Big Canoe Engineer of Record
- 6. Georgia Department of Natural Resources (GA DNR)
- 7. Georgia Safe Dams Program
- 8. Pickens County Sheriff
- 9. Pickens County Public Works
- 10. Cox Lake Dam Owner, Cove Lake Property Association

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

Date:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

Date:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.	
Organization:	
Name:	
Title:	840
Signature:	
Date:	

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake Dam and understand the agreed-upon responsibilities.

Organization:	
EPD- GEORGIA SAFE DAMS PROGRAM	
Name: Tom Woosley	±
Title: PROGRAM MANAGEL	я
Signature: John Won	
Date: 812017	

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.
Organization:
Name:
Title:
Signature:
Date:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.
Organization:
Name:
Title:
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Date:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake

Dam and understand the agreed-upon responsibilities.

Organization:

Name:

Title:

Signature:

By my signature, I acknowledge that I am aware of the Emergency Action Plan for Petit Lake		
Dam and understand the agreed-upon responsibilities.		
Organization:		
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Signature:		
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APPENDIX E EAP REVIEW, REVISIONS, AND TESTING DOCUMENTATION

APPENDIX E

EAP REVIEW, REVISION, AND PERIODIC TEST

EAP Annual Review

An Annual Review should be conducted by the Big Canoe Property Owner's Association. The annual review should be documented in this appendix.

EAP Periodic Test

A periodic test of the EAP procedures is recommended every 5 years. Documentation for the test is provided in this appendix.

Revision

Revisions required from annual reviews or periodic testing should be documented in this appendix. Revisions shall be made in all copies of the EAP as provided in Appendix D.

RECORD OF ANNUAL REVIEW

Date	Conducted By	Notes and Observations
	a a	40
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v		ω .

RECORD OF PERIODIC TESTING

Date	Conducted By	Observations
	<u> </u>	
150	*	

RECORD OF REVISIONS AND UPDATES

Revision Number	Date	Revisions Made
0	September 1998	Original Issue
1	April 2007	Contact information update
2	June 2017	Contact Information, Added Condition C, Updated Appendices
3	May 2018	Update Condition Identifications and added Table 3
	-	
	24	
<u>0</u>		

APPENDIX F LOCALLY AVAILABLE RESOURCES

APPENDIX F

LOCALLY AVAILABLE RESOURCES

The Pickens County Commissioner indicated the following heavy equipment may be available in the case of an emergency (for use, contact John Nicholson at Pickens County EMA at 706-253-8829 or Kim Quinton at the Pickens County Public Works Department at 770-893-9564):

- One (1) Caterpillar D6 Bulldozer;
- One (1) 330 Kobelco Excavator;
- One (1) Kubota 080 Excavator;
- One (1) Caterpillar 303 Excavator;
- One (1) Caterpillar 313 Rubber Tire Trackhoe;
- Two (2) Bobcat skid steers;
- Three (3) tandem dump trucks;
- Two (2) single axle box dump trucks; and
- Three (3) single axle flat bed trucks.

Other locally available resources include:

Heavy Equipment Service and Rental	Sand and Gravel Supply	Ready-mix Concrete Supply
Sunbelt Rentals 5290 Lake Pointe Center Drive Cumming, Georgia Main: 770-887-9966 Emergency: 800-667-9328 United Rentals 1151 Northpoint Parkway SE Acworth, Georgia 30102 Main: 770-974-3500 (On call service available 24/7)	Bluegrass Quarry 970 Old Nelson Road Ball Ground, Georgia 30107 678-641-7714 Vulcan Materials Company 4420 Hightower Road Ball Ground, Georgia 30107 Main: 678-947-3310 GA Services: 770-454-3691	Wayne Davis Concrete 115 River Mill Drive Ball Ground, Georgia 706-692-3464 Ernst Concrete 970 Old Nelson Road Ball Ground, Georgia 30107 770-853-0533 Argos Ready Mix 829 Univeter Road Canton, Georgia 770-704-7778
Pumps	Diving Service	Sand Bags
Xylem Cartersville 402 Old Mill Road Cartersville, Georgia 30120 770-415-8814 (On call service available 24/7) United Rentals 5260 Truman Drive Decatur, Georgia 30035 Main: 404-439-4322 (On call service available 24/7)	Georgia Department of Natural Resources, Law Enforcement Division, Search and Rescue Team 781 Red Top Mountain Road SE, Acworth, Georgia 30102 770-529-2424 (M-F 8am – 4:30pm) 1-800-241-4113 (After hours) Underwater Construction Corporation 8494 Gulf View Drive Soddy Daisy, TN 37379 423-332-6700	Hanes Geo Components 3105 Sweetwater Road, Shite 200 Lawrenceville, Georgia 30044 Main: 866-961-3565 Emergency: 678-221-7849